

COMUNE DI CAMPOGALLIANO (MO)

PROGETTO PER LA REALIZZAZIONE DI NUOVI DEPOSITI E RELATIVI UFFICI

ACCORDO OPERATIVO

TAVOLA

Microzonizzazione sismica

Data: 24/07/20

Scala:

COMMITTENTE

Logistica e Sviluppo s.r.l.

Logistica e sviluppo s.r.l.

Via Strasburgo,31
41011 Campogalliano (MO)

GENERAL CONTRACTOR



GSE Italia s.r.l.

Via G.B. Percolesi, 27
20124 Milano (MI)

PROGETTISTA E DL



Solo Filippi - Services For Your Estate

ING. FILIPPO SALIS

Via della Moscova, 47 - 20121 Milano
Iscrizione ordine ingegneri Milano: MI 24867 - I - 2961

CO-PROGETTISTA URBANISTICO

Studio Dott. Ing. Grotti Loris

Dott. Ing. Grotti Loris

Via Vignolese n. 1389
41126 San Damaso (MO)

Commessa

1

4

9

3

3

Fase

R

T

Lotto

Disegno

07

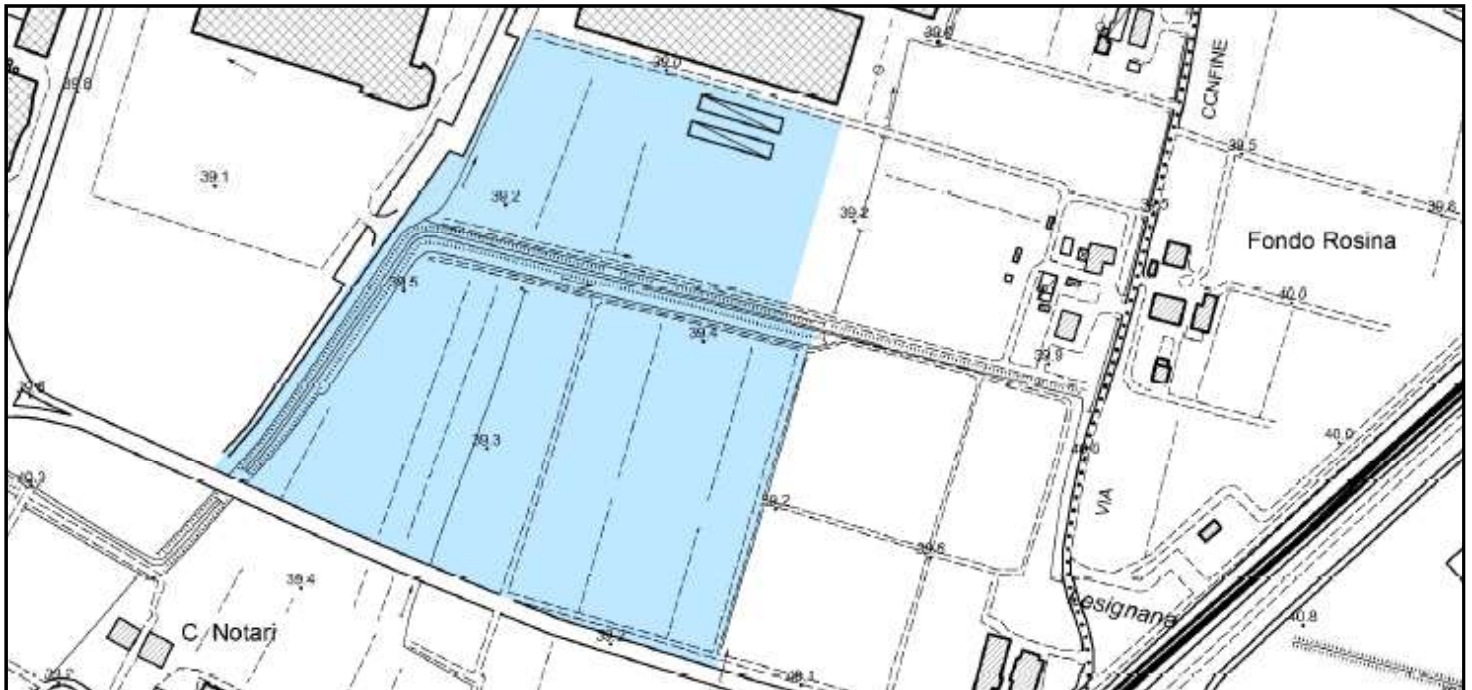
Rev

A

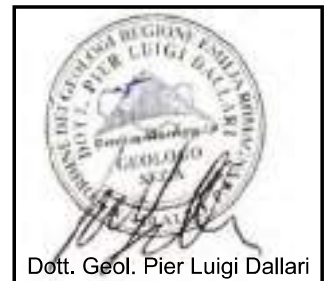
COMUNE DI CAMPOGALLIANO

PROVINCIA DI MODENA

Integrazione alla relazione contenente lo studio di **“MICROZONAZIONE SISMICA DEL SITO”** (ai sensi della DGR 630/2019 RER) con verifica del coefficiente di amplificazione del moto sismico secondo approccio descritto nella normativa della Regione Emilia Romagna, verifica dei fenomeni di liquefazione e dei cedimenti post-sismici.



INTEGRAZIONE ALLA RELAZIONE DI MICROZONAZIONE SISMICA (RIF. 483_2019) PER IL PROGETTO PER LA REALIZZAZIONE DI UN NUOVO DEPOSITO



Dott. Geol. Pier Luigi Dallari

GIUGNO 2020
Rif. 308/20



UFFICI: via Per Modena, 12 - 41051 Castelnuovo R. (MO) SEDE LEGALE: via C.Costa, 182 - 41123 Modena (MO)
tel. 059/3967169; 059/828367 - FAX 059/5960176 p. Iva e C.F.02981500362
Email: info@geogroupmodena.it Pec: geo.group@winpec.it Sito: www.geogroupmodena.it
Operatore APR certificato ENAC per operazioni non critiche – Rif. ENAC 18627

COMUNE DI CAMPOGALLIANO

PROVINCIA DI MODENA

**RELAZIONE GEOLOGICA GEOTECNICA E SISMICA INERENTE L'INTEGRAZIONE
RICHIESTA DALLA PROVINCIA DI MODENA IN DATA 08/05/2020 PER LA PROPOSTA DI
ACCORDO OPERATIVO AI SENSI DELL' ARTICOLO N. 38 DELLA RER 24/17 SUGLI
AMBITI ELEMENTARI 45.35-PARTE E 47.32 PER L'AMPLIAMENTO DELLA DITTA
TRANSMEC SPA**

Rif. n. 308/2020

LOCALITÀ

Comune di Campogalliano
Provincia Modena

COMMITTENTE

Transmec Group – Trasporti Internazionali Transmec spa



UFFICI: via Per Modena, 12 - 41051 Castelnuovo I
tel. 059/396716

Email: info@geogroupmode

Operatore APR certificato ENAC per operazioni non critiche – Rif. ENAC 18627



INDICE DEL CONTENUTO

1. PREMESSE	3
2. INQUADRAMENTO GENERALE	4
2.1 Aspetti geologici generali	4
2.2 Microzonazione sismica Comune di Campogalliano	6
3. INDAGINI GEOGNOSTICHE	7
3.1 Campagna geognostica	7
4. STABILITÀ DEL SITO NEI CONFRONTI DEI FENOMENI CO-SISMICI	10
4.1 Verifica della suscettività del sito ai fenomeni di liquefazione (Approccio DGR 630/19)	10
5.0 Verifica dei cedimenti post-sismici (DGR 630/2019)	12
6 CONSIDERAZIONI CONCLUSIVE	15

TAVOLE

Tav. n. 1: “Carta topografica”	scala 1: 3000;
Tav. n. 2: “Ripresa fotografica aerea”	scala 1: 3000;
Tav. n. 3: “Carta geologica”	scala 1:5000;
Tav. n. 4: “Ubicazione delle indagini”	scala grafica;

ALLEGATI

- ALL. n. 1 Prove penetrometriche statiche con punta elettrica e piezocono CPTU;
- ALL. n. 2 Verifiche della suscettività del sito al fenomeno di liquefazione.



1. PREMESSE

Nel mese di Giugno 2020 è stata redatta la seguente integrazione su richiesta della Provincia di Modena inerente la proposta di accordo operativo ai sensi dell' articolo 38 della L.R. 24/17 sugli ambiti elementari 45.35 – parte e 47.32, per l'ampliamento della ditta *Transmec spa* in Via Per Modena all'interno del territorio del Comune di Campogalliano (MO).

In precedenza, nel mese di agosto 2019, era stata eseguita l'analisi di pericolosità sismica locale in prospettiva di microzonazione sismica a supporto della progettazione urbanistica per la progettazione di un nuovo deposito, secondo le disposizioni indicate dalla normativa regionale DGR 630/19.

La seguente integrazione è stata espletata nel mese di giugno 2020 mediante n. 3 prove penetrometriche statiche con punta elettrica e piezocono CPTu, e come richiesto dall'ufficio di pianificazione urbanistica territoriale e cartografica della Provincia di Modena è stato verificato il rischio di potenziale liquefazione per le verticali di prova delle nuove CPTu eseguite e la verifica dei cedimenti post sismici indotti dall'azione sismica secondo le vigenti normative regionali DGR n. 630 del 29/04/2019.

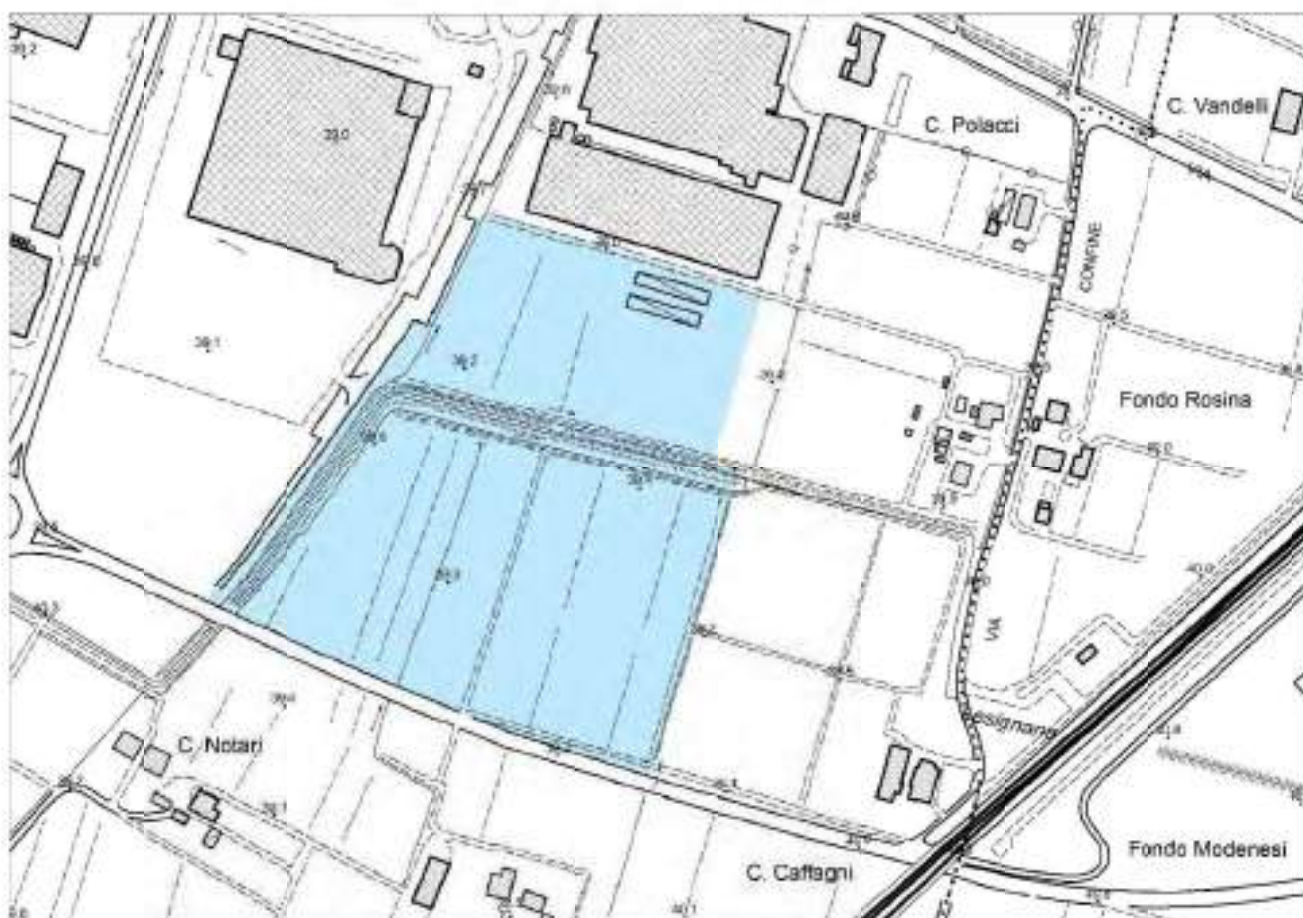


Fig. n. 1.1 – Cartografia tecnica regionale con indicazione dell'area di interesse

2. INQUADRAMENTO GENERALE

2.1 Aspetti geologici generali

L'area in analisi è sita nella bassa pianura modenese, in un contesto geologico superficiale di inter-conoide alluvionale riconducibile alle trasgressioni di aste fluviali minori. Secondo quanto illustrato nella cartografia geologica contenuta nello studio di microzonazione sismica del Comune di Campogalliano il sottosuolo risulta principalmente costituito da sabbie argillose e miscele di sabbia e argilla.

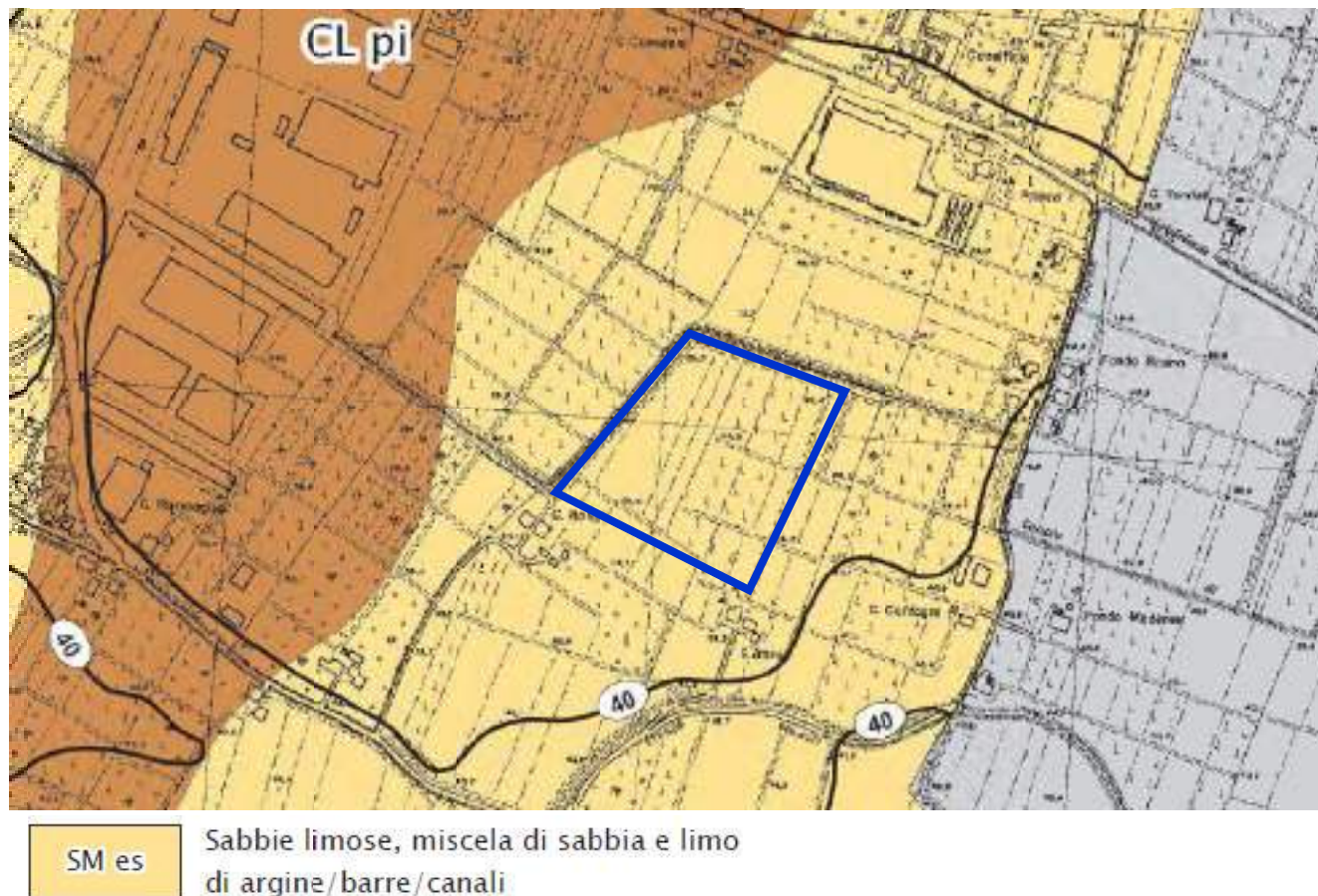


Fig. 2.1 – Estratto della cartografia "CARTA GEOLOGICO-TECNICA" allegata allo studio di microzonazione sismica del Comune di Campogalliano

La conformazione del substrato in corrispondenza dell'area di interesse è interessata dalla presenza di lembi rialzati dovuti a sovrascorrimenti del periodo post-tortoniano (Miocene) (Fig. n. 2.2). Per quanto riguarda la profondità dei depositi superficiali che costituiscono il Sistema Emiliano Romagnolo Superiore, dalla consultazione dei dati bibliografici a disposizione, presso l'area in analisi risulta corrispondere alla quota di -3.40 m (Fig. 2.3) rispetto al livello medio del mare.

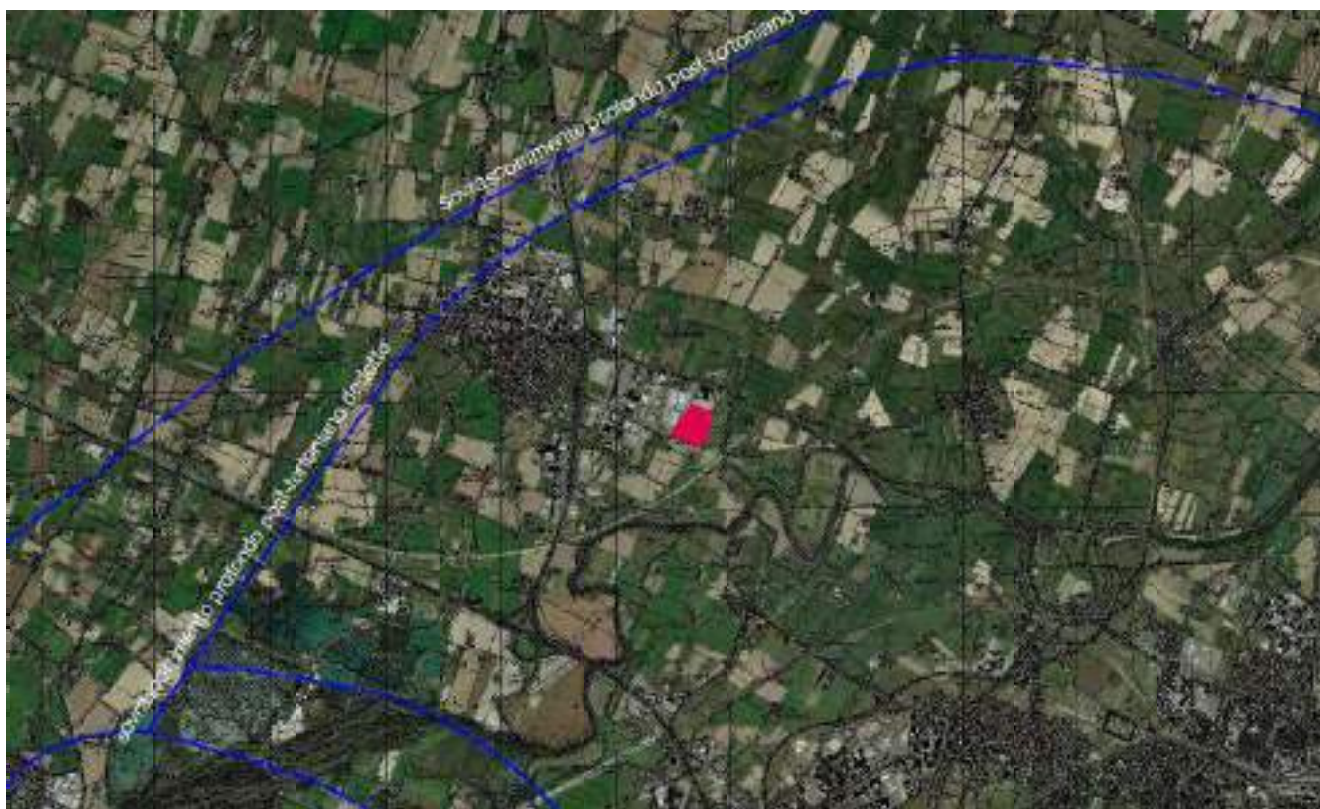


Fig. 2.2 – Rappresentazione dell'assetto delle unità di substrato (fonte: RER);

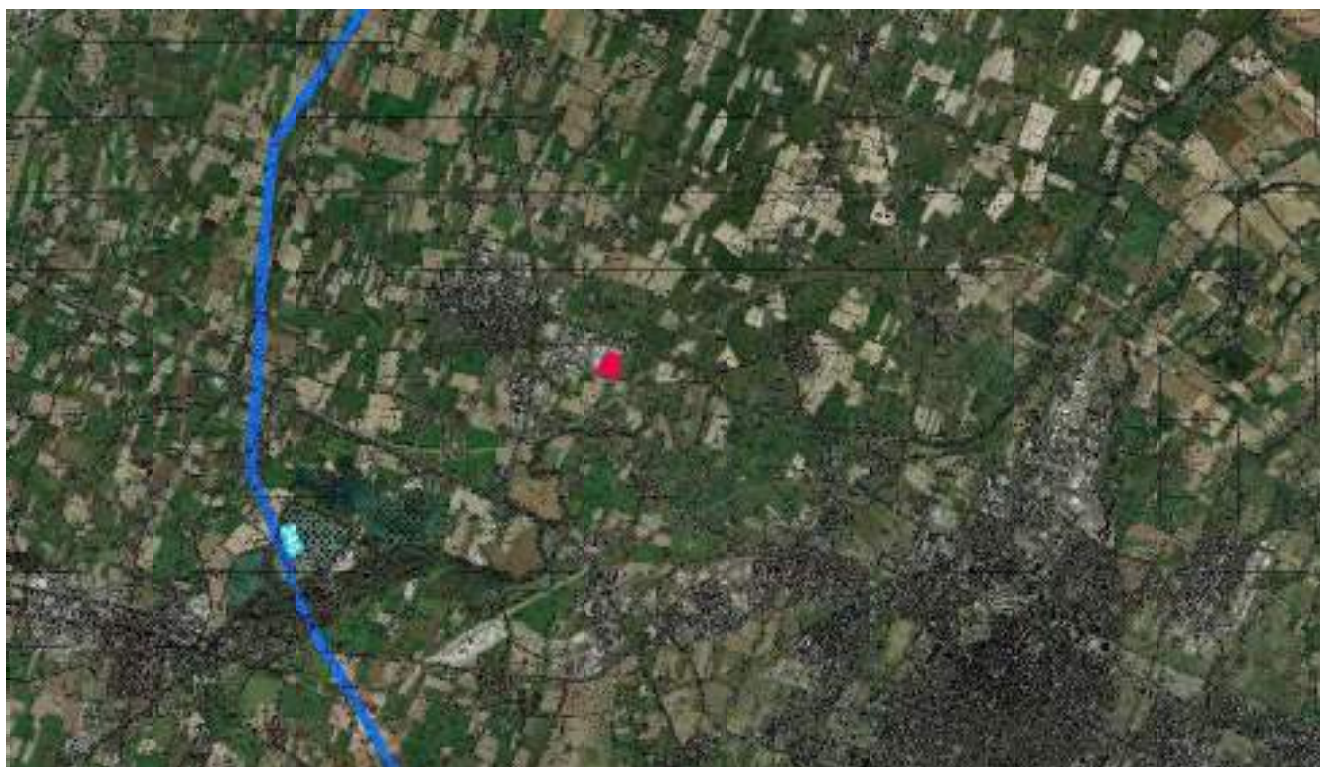
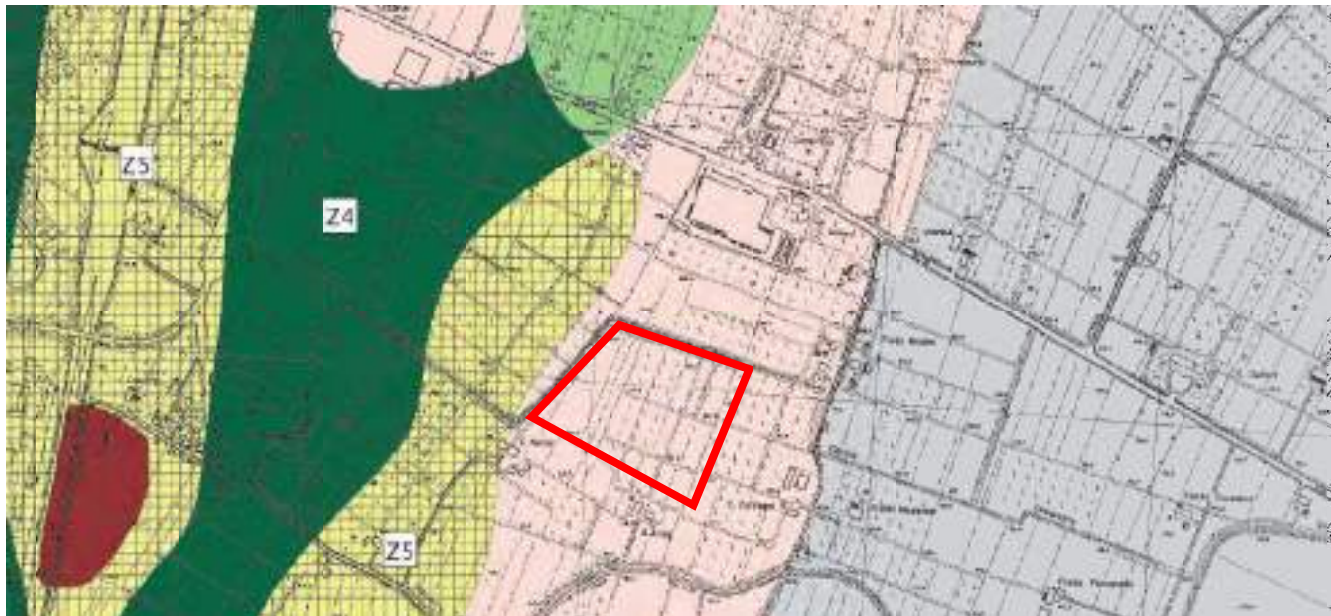


Fig. 2.3 – Isobate della base del Sistema Emiliano Romagnolo Superiore (fonte: RER);

2.2 Microzonazione sismica Comune di Campogalliano

A monte della fase di modellazione diretta delle caratteristiche geologiche e sismiche a corredo del presente studio MS, sono stati consultati gli elaborati cartografici relativi allo studio MS relativo al Comune di Campogalliano (Aprile 2013).

Consultando la cartografia MOPS di II° livello (Carta delle microzone omogenee in prospettiva sismica) (Fig. 2.4) l'area in esame ricade in corrispondenza di zone suscettibili di instabilità per potenziali cedimenti differenziali e addensamenti.



Zone stabili suscettibili di instabilità



Z4a: Zona di attenzione per liquefazione tipo I



POTENZIALI CEDIMENTI DIFFERENZIALI

Lim. - limi argilloso sabbiosi poco consistenti (CL / ML - Z5) contenenti corpi sabbiosi limosi limoforni, con spessori inferiori a 1,0 m da poco addensati, poco consistenti a moderatamente addensati (SM - 13/12); moderatamente consistenti (ML-24) nei primi 12 m dal p.c.

Argille limose - limi argillosi poco consistenti (ML / CL - Z5) con intercalati corpi limoforni sabbioso limosi, con spessori inferiori a 0,5 m da moderatamente a poco addensati (SM 12/13) nei primi 12-13 m p.c.

Effetti attesi: amplificazione per caratteristiche litostratigrafiche, cedimenti differenziali, addensamenti; Approfondimenti di III° livello per stima dei cedimenti differenziali.

Fig. 2.4 – Estratto della "Carta delle microzone omogenee in prospettiva sismica" del Comune di Campogalliano

Considerato l'assetto appena illustrato si rende necessaria l'esecuzione di specifiche analisi secondo III livello di approfondimento per la determinazione dell'azione sismica e del relativo rischio di liquefazione.

3. INDAGINI GEOGNOSTICHE

3.1 Campagna geognostica

Con la finalità di integrare il precedente studio di microzonazione sismica e ottemperare alle richieste della normativa regionale DGR 630 del 29/04/2019, in data 11-06-2020, sono state eseguite ulteriori indagini riassunte nella successiva tabella:

n. 2 prove penetrometriche statiche con punta elettrica e piezocono CPTu	anno 2014
n. 7 prove penetrometriche statiche con punta meccanica CPT	anno 2014
n. 2 indagini sismiche passive con metodologia HVSr	anno 2014
n. 1 indagine sismica passiva con metodologia RE.MI	anno 2014
n. 1 indagine sismica attiva con metodologia MASW	anno 2014
n. 4 prove penetrometriche statiche con punta meccanica CPT	anno 2017
n. 3 prove penetrometriche statiche con punta elettrica e piezocono CPTu	anno 2020

Nella seguente Fig. 3.1 è illustrata la planimetria di dettaglio e l'ubicazione delle indagini geognostiche precedenti e delle nuove indagini **CPTu integrative (in giallo)**, eseguite in data 11-06-2020. Si procede, nel capitolo successivo, a descrivere le metodologie di indagine e analisi adottate a supporto della redazione del presente documento.

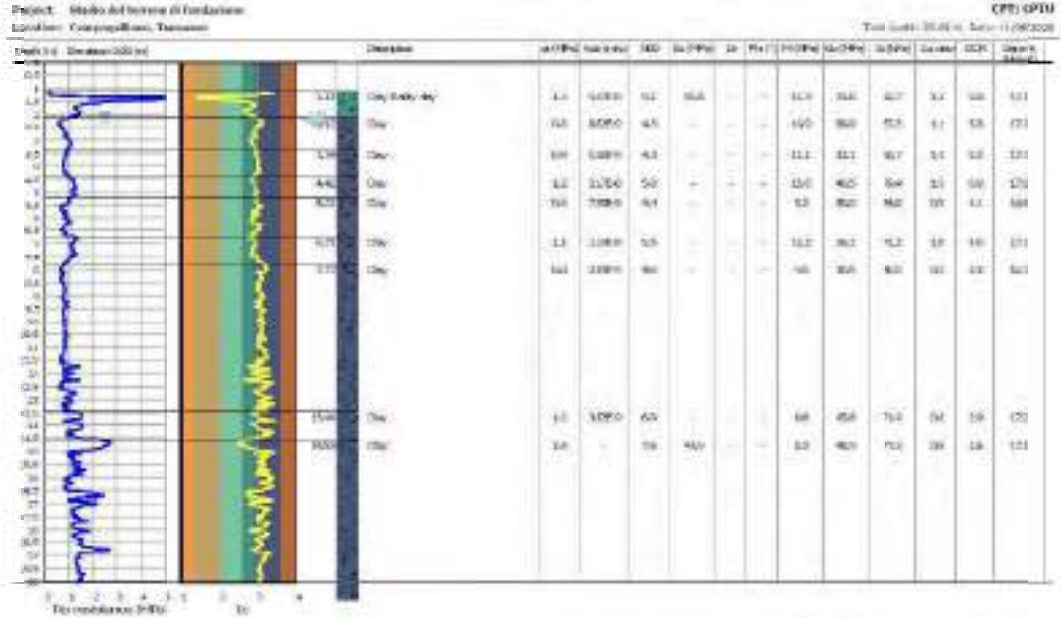




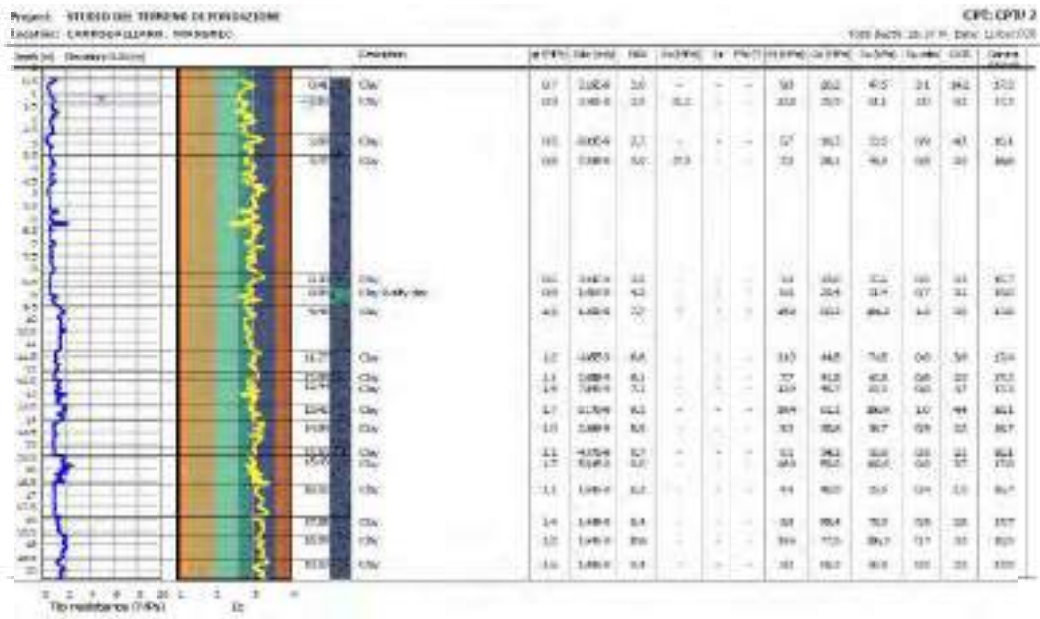
● Prove penetrometriche statiche CPTU eseguite in data
11-06-2020

Fig. n. 3.1 – Ubicazione delle indagini geognostiche

CPTU 1



CPTU 2



UFFICI: via Per Modena, 12 - 41051 Castelnovo R. (MO) SEDE LEGALE: via C.Costa,182 - 41123 Modena (MO)
tel. 059/3967169; 059/828367 - FAX 059/5960176 p. Iva e C.F.02981500362
Email: info@geogroupmodena.it Pec: geo.group@winpec.it Sito: www.geogroupmodena.it
Operatore APR certificato ENAC per operazioni non critiche - Rif. ENAC 18627

CPTU 3

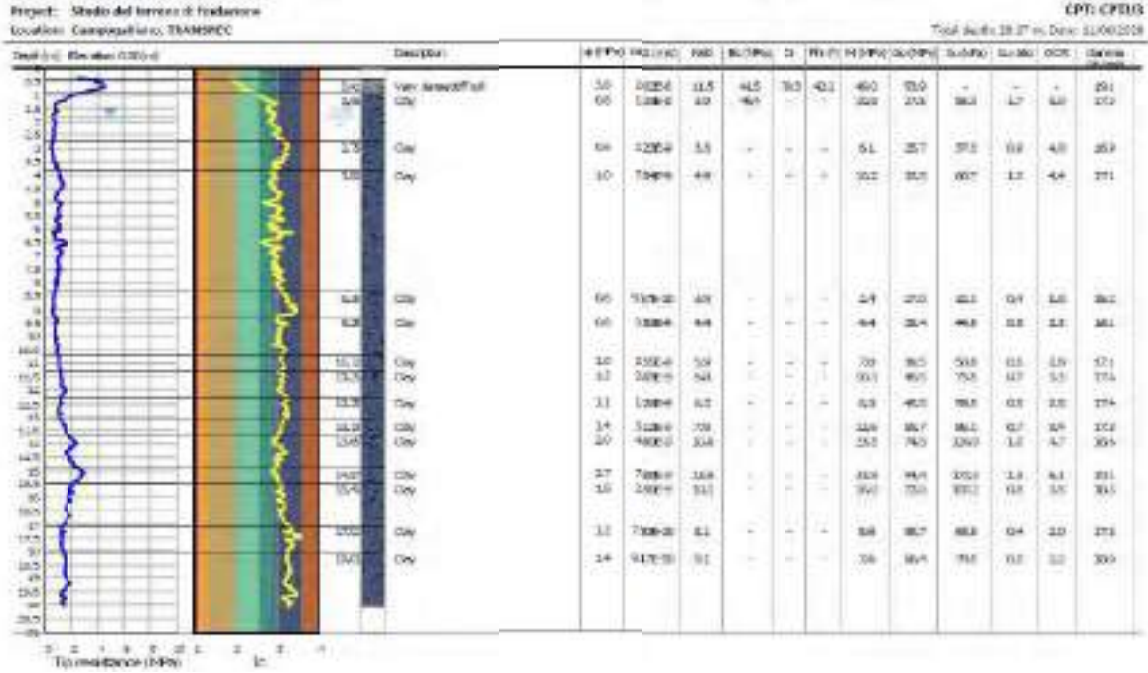


Fig. n. 3.2 – Sezioni geotecniche medie delle CPTu eseguite

4. STABILITÀ DEL SITO NEI CONFRONTI DEI FENOMENI CO-SISMICI

4.1 Verifica della suscettività del sito ai fenomeni di liquefazione (Approccio DGR 630/19)

In accordo con la normativa regionale DGR 630/19 si è proceduto alla verifica della liquefazione per la totalità delle verticali di prova CPTU eseguite in data 11-06-2020.

La valutazione del fenomeno della liquefazione viene svolta attraverso il calcolo del fattore di sicurezza di ciascun livello che compone il sottosuolo analizzato nei confronti del fenomeno stesso. Il fattore di sicurezza è derivato secondo la seguente formulazione:

$$F_L(z) = \frac{CRR_{M=7.5, \sigma_v = 1 \text{ atm}}}{CSR} \cdot MSF \cdot K_\sigma$$

Dove:

CRR è rappresentato dal rapporto di resistenza ciclica

MSF rappresenta il fattore di scala della magnitudo che è funzione della magnitudo stessa, della PGA e del valore di CRR



K_σ è il fattore di correzione che tiene conto della pressione efficace alla profondità a cui la resistenza viene valutata

CSR è il rapporto di tensione ciclica, ovvero la tensione di taglio indotta dall'azione sismica, normalizzata rispetto alla tensione verticale efficace. Tale parametro è derivabile secondo la seguente formulazione:

$$CSR = \frac{\tau_{media}}{\sigma_{v0}} = 0.65 \cdot \frac{a_{max}}{g} \cdot \frac{\sigma_{v0}}{\sigma_{v0}} \cdot r_d$$

i termini che compongono la seguente relazione sono rappresentati da a_{max}/g che descrive il valore dell'accelerazione orizzontale a $T = 0.00$ s, il rapporto della tensione litostatica totale ed efficace, e il coefficiente riduttivo r_d . Quest'ultimo, in accordo con la predetta normativa è stato stimato secondo la seguente formulazione:

$$r_d = \exp[\alpha(z) + \beta(z) \cdot M]$$

$$\alpha(z) = -1.012 - 1.126 \cdot \text{sen}\left(\frac{z}{11.73} + 5.133\right)$$

$$\beta(z) = 0.106 + 0.118 \cdot \text{sen}\left(\frac{z}{11.28} + 5.142\right)$$

dove M rappresenta il valore di magnitudo di riferimento per il sito in analisi.

Il valore **CRR** è stato ricavato attraverso la metodologia di calcolo proposta da **Boulanger&Idriss 2014**. Per le specifiche della procedura di calcolo adottata si faccia riferimento all'allegato **A2.2** della DGR 630/2019 della Regione Emilia Romagna. Le analisi di liquefazione sono state eseguite entro i -20.00 m rispetto al piano campagna come prescritto dalla normativa di riferimento.

Le verifiche sono state svolte considerando una magnitudo di riferimento **M = 6.14** (derivante dalla zonazione sismogenetica), una accelerazione massima al suolo **A_{max} = 0.236 g** (Approccio semplificato NTC 2018 Categoria di sottosuolo C $V_n=50$ anni e $C_u = 1.00$) e con soggiacenza della falda freatica alla profondità di $D_w = -1.00$ m da p.c., che rappresenta un valore di soggiacenza cautelativo alla luce dei valori di soggiacenza riscontrati durante l'esecuzione delle indagini.

A seguito dell'identificazione del fattore di sicurezza F_L è stato determinato l'indice potenziale di liquefazione IL (LPI) attraverso il quale è possibile definire l'esposizione del sito al rischio di liquefazione. Il potenziale di liquefazione LPI è stato determinato secondo la seguente formulazione:

$$I_L = \int_0^{z_{crit}} F(z) \cdot w(z) \cdot dz \quad \text{in cui} \quad w(z) = \frac{200}{z_{crit}} \cdot \left(1 - \frac{z}{z_{crit}}\right)$$

Dove $F(z)$ viene determinato secondo le formulazioni proposte da Somnez (2003)

$F(z) = 0$	per	$F_L \geq 1.2$
$F(z) = 2 \cdot 10^6 \cdot \exp(-18.427 \cdot F_L)$	per	$1.2 > F_L \geq 0.95$
$F(z) = 1 - F_L$	per	$F_L \leq 0.95$



In base al valore di LPI ottenuto è possibile fornire un'indicazione del rischio di liquefazione. In considerazione delle linee guida emesse dalla Regione Emilia Romagna, a seguito degli eventi sismici di Maggio e Giugno 2012, si considera la classificazione di rischio definita da **Sonmez 2003**:

INDICE DI LIQUEFAZIONE	Rischio di liquefazione
LPI=0	Nulla
0 < LPI ≤ 2	Basso
2 < LPI ≤ 5	Moderato
5 < LPI ≤ 10	Alto
15 > LPI	Molto alto

Si illustrano successivamente i valori di LPI derivati per ciascuna verticale di prova CPTU considerata ai fini delle analisi del fenomeno di liquefazione:

CPTu 11-06-2020		
INDAGINE	INDICE DI LIQUEFAZIONE LPI	RISCHIO
CPTU 1	0.270	BASSO
CPTU 2	0.277	BASSO
CPTU 3	0.0	NULLO

Tab. n. 4.1.1 – Valori del potenziale di liquefazione ottenuti dalle CPTu

Sulla scorta delle risultanze ottenute dalle verifiche della suscettività del sito al fenomeno di liquefazione emerge che l'areale oggetto di studio risulta esposto a un rischio "basso".

5.0 Verifica dei cedimenti post-sismici (DGR 630/2019)

Per cedimento si intende l'abbassamento del piano campagna, ovvero la sommatoria dei prodotti della deformazione verticale media per lo spessore di ciascuno degli strati deformati.

Il cedimento post sismico può essere dovuto a terreni granulari saturi, insaturi e a terreni coesivi soffici; tali cedimenti vengono stimati con la seguente equazione:

$$s = \sum_{i=1}^n \varepsilon_{vi} \cdot \Delta z_i$$

ε_{vi} = deformazione volumetrica;

Δz_i = spessore dello strato i-esimo.

I cedimenti conseguenti a liquefazione di terreni granulari saturi hanno luogo immediatamente dopo il terremoto o comunque in un tempo non superiore a un giorno, i cedimenti per densificazione di terreni granulari insaturi avvengono durante lo scuotimento sismico, mentre i cedimenti per consolidazione di terreni coesivi soffici possono richiedere tempi anche molto lunghi in funzione delle condizioni stratigrafiche e geotecniche.

Il calcolo dei cedimenti post sismici in terreni incoerenti in condizioni saturate e insature, eseguiti per la verticale CPTU ha evidenziato quanto segue:

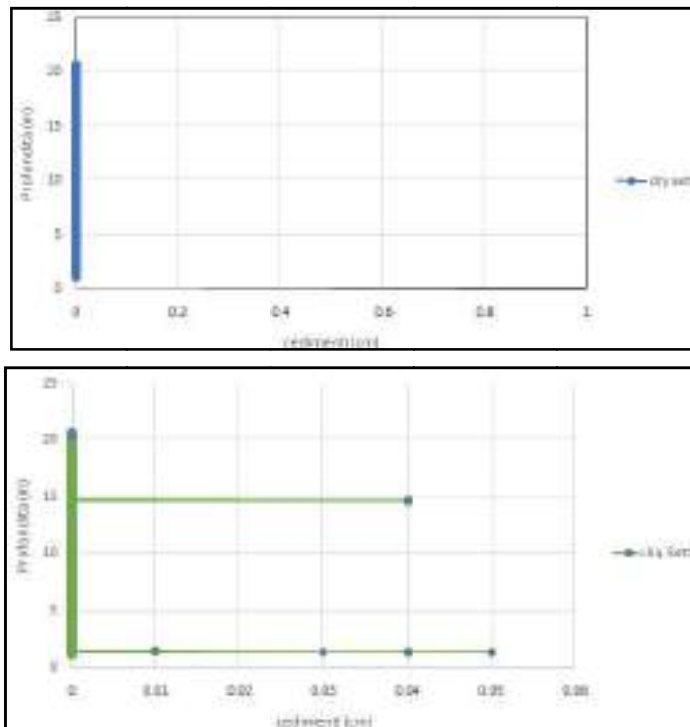


Si riportano di seguito i valori calcolati dei cedimenti post sismici in terreni incoerenti sia in saturi (Liq. seattlement) che insaturi (Dry settlement) per le verticali CPTu eseguite.

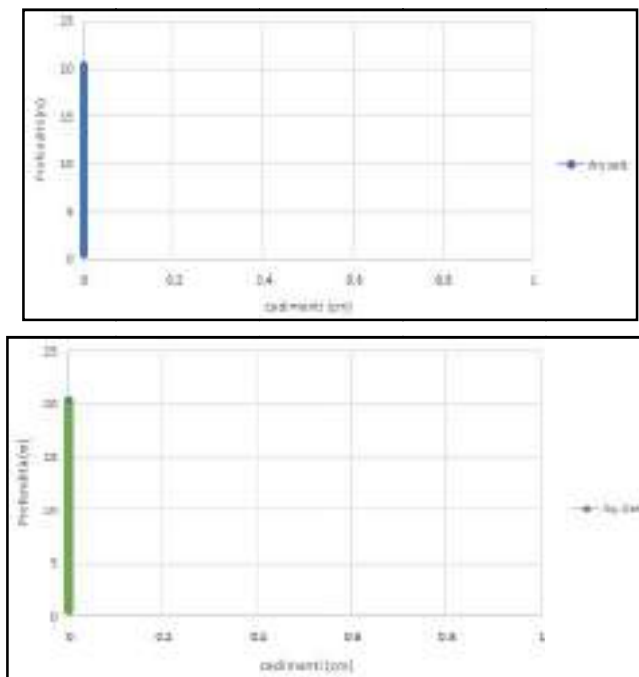
CPTu 1	
CEDIMENTO IN TERRENI INCOERENTI SATURI – (Liq. Seattle)	0.6 cm
CEDIMENTO IN TERRENI INCOERENTI INSATURI – (Dry. Seattle)	0.0 cm
CPTu 2	
CEDIMENTO IN TERRENI INCOERENTI SATURI – (Liq. Seattle)	0.0 cm
CEDIMENTO IN TERRENI INCOERENTI INSATURI – (Dry. Seattle)	0.0 cm
CPTu 3	
CEDIMENTO IN TERRENI INCOERENTI SATURI – (Liq. Seattle)	0.0 cm
CEDIMENTO IN TERRENI INCOERENTI INSATURI – (Dry. Seattle)	0.0 cm

Tab. n.5.1 – Valori dei cedimenti post-sismici ottenuti dalle CPTu

CPTu 1



CPTu 2



CPTu 3

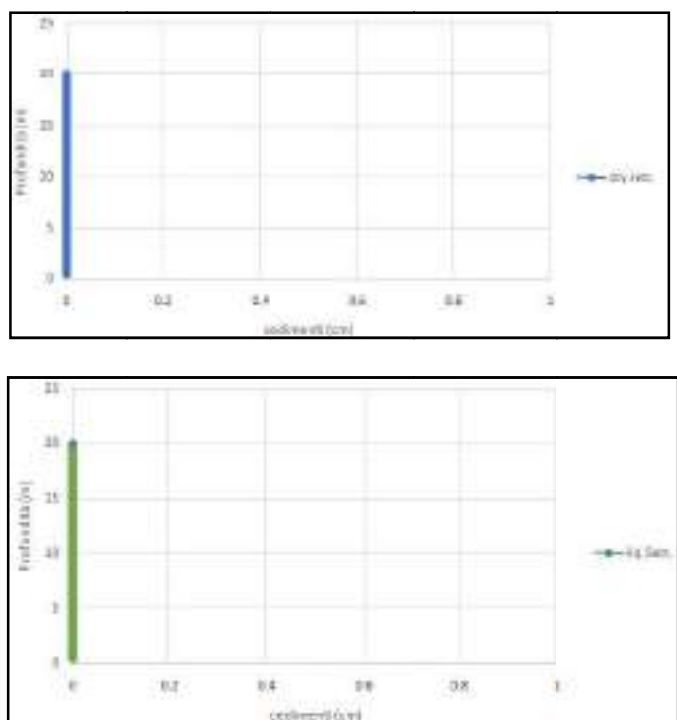


Fig.5.2 –Cedimenti post sismici in terreni incoerenti saturi (Liquef. Settlement) e in terreni incoerenti insaturi (Dry settlement) eseguiti per le prove CPTU.

I cedimenti calcolati sono da intendersi verificabili solamente al termine di un evento sismico avente magnitudo e accelerazioni comparabili a quelle utilizzate per le analisi descritte.

6 CONSIDERAZIONI CONCLUSIVE

La presente integrazione allo studio di Microzonazione Sismica è stata espletata mediante l'esecuzione di n. 3 prove penetrometriche con punta elettrica e piezocono CPTu Spinte a -20.00 m cadauna in data 11/06/2020, mediante le verifiche di potenziale di liquefazione e il calcolo dei cedimenti indotti dall'azione sismica come da richiesta dell'ufficio Pianificazione Urbanistica Territoriale e Cartografica U.o. Difesa del suolo della Provincia di Modena.

Per determinare le verifiche di potenziale liquefazione è stato utilizzato il metodo di calcolo di Idriss Boulanger 2014 con la seguente formula:

$$I_L = \int_0^{z_{crit}} F(z) \cdot w(z) \cdot dz \quad \text{in cui} \quad w(z) = \frac{200}{z_{crit}} \cdot \left(1 - \frac{z}{z_{crit}}\right)$$

mentre per calcolare i cedimenti post sismici è stata utilizzata la seguente equazione:

$$s = \sum_{i=1}^n \varepsilon_{vi} \cdot \Delta z_i$$

dove

ε_{vi} = deformazione volumetrica;

Δz_i = spessore dello strato i-esimo.

Tali verifiche sono state eseguite in ottemperanza alla DGR 630/2019 della Regione Emilia Romagna.

Si riassumono successivamente i valori di LPI (Indice potenziale di liquefazione) e cedimenti indotti dal sisma derivati:

CPTu 11-06-2020		
INDAGINE	INDICE DI LIQUEFAZIONE LPI	RISCHIO
CPTU 1	0.270	BASSO
CPTU 2	0.277	BASSO
CPTU 3	0.0	NULLO

Tab.6.1 – Valori del potenziale di liquefazione ottenuti dalle CPTu



CPTu 1	
CEDIMENTO IN TERRENI INCOERENTI SATURI – (Liq. Seattle)	0.6 cm
CEDIMENTO IN TERRENI INCOERENTI INSATURI – (Dry. Seattle)	0.0 cm
CPTu 2	
CEDIMENTO IN TERRENI INCOERENTI SATURI – (Liq. Seattle)	0.0 cm
CEDIMENTO IN TERRENI INCOERENTI INSATURI – (Dry. Seattle)	0.0 cm
CPTu 3	
CEDIMENTO IN TERRENI INCOERENTI SATURI – (Liq. Seattle)	0.0 cm
CEDIMENTO IN TERRENI INCOERENTI INSATURI – (Dry. Seattle)	0.0 cm

Tab. n.6.2 – Valori dei cedimenti post-sismici ottenuti dalle CPTu

Sulla scorta degli esiti appena illustrati è possibile definire l'assenza di particolari criticità riconducibili ad una potenziale instabilità del sito per effetti sismici, differentemente da quanto indicato nella cartografia MOPS relativa allo studio MS del Comune di Campogalliano.

In relazione alle verifiche eseguite è possibile affermare che il sito di interesse risulta privo di potenziale instabilità alla liquefazione per effetto sismico ed i cedimenti post sismici calcolati sulle verticali delle n. 3 prove penetrometriche CPTu, eseguite in data 11/06/2020, hanno fornito valori molto modesti e ammissibili per le strutture in progetto.

Si precisa inoltre che la progettazione antisismica esecutiva potrà essere condotta secondo approccio semplificato NTC 2018.

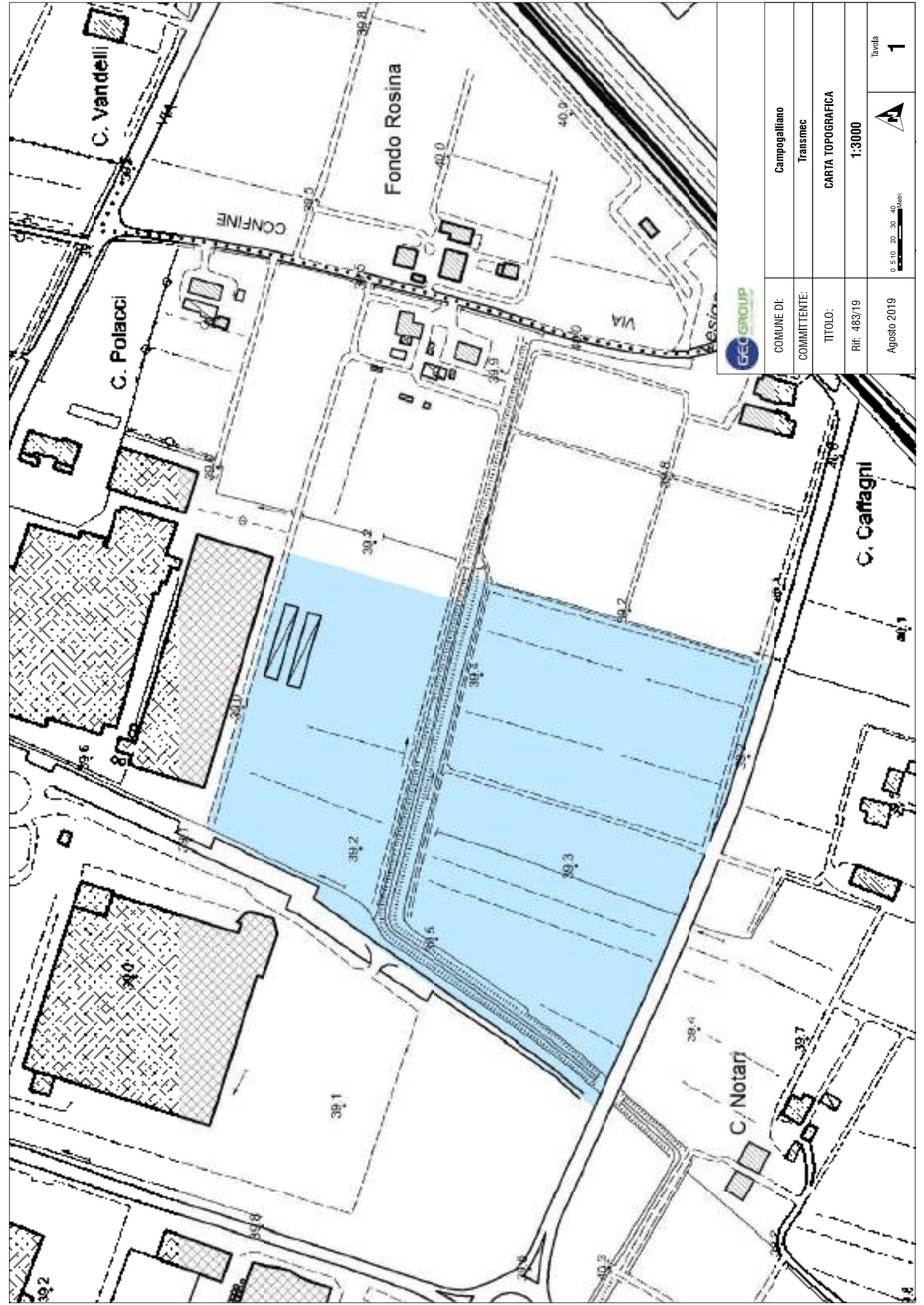
Modena, 12 Giugno 2020
Dott. Geol. Pier Luigi Dallari
GEO GROUP SRL







UFFICI: via Per Modena, 12 - 41051 Castelnuovo R. (MO) SEDE LEGALE: via C. Costa, 182 - 41123 Modena (MO)
tel. 059/3967169; 059/828367 - FAX 059/5960176 p. Iva e C.F. 02981500362
Email: info@geogroupmodena.it Pec: geo.group@winpec.it Sito: www.geogroupmodena.it
Operatore APR certificato ENAC per operazioni non critiche – Rif. ENAC 18627

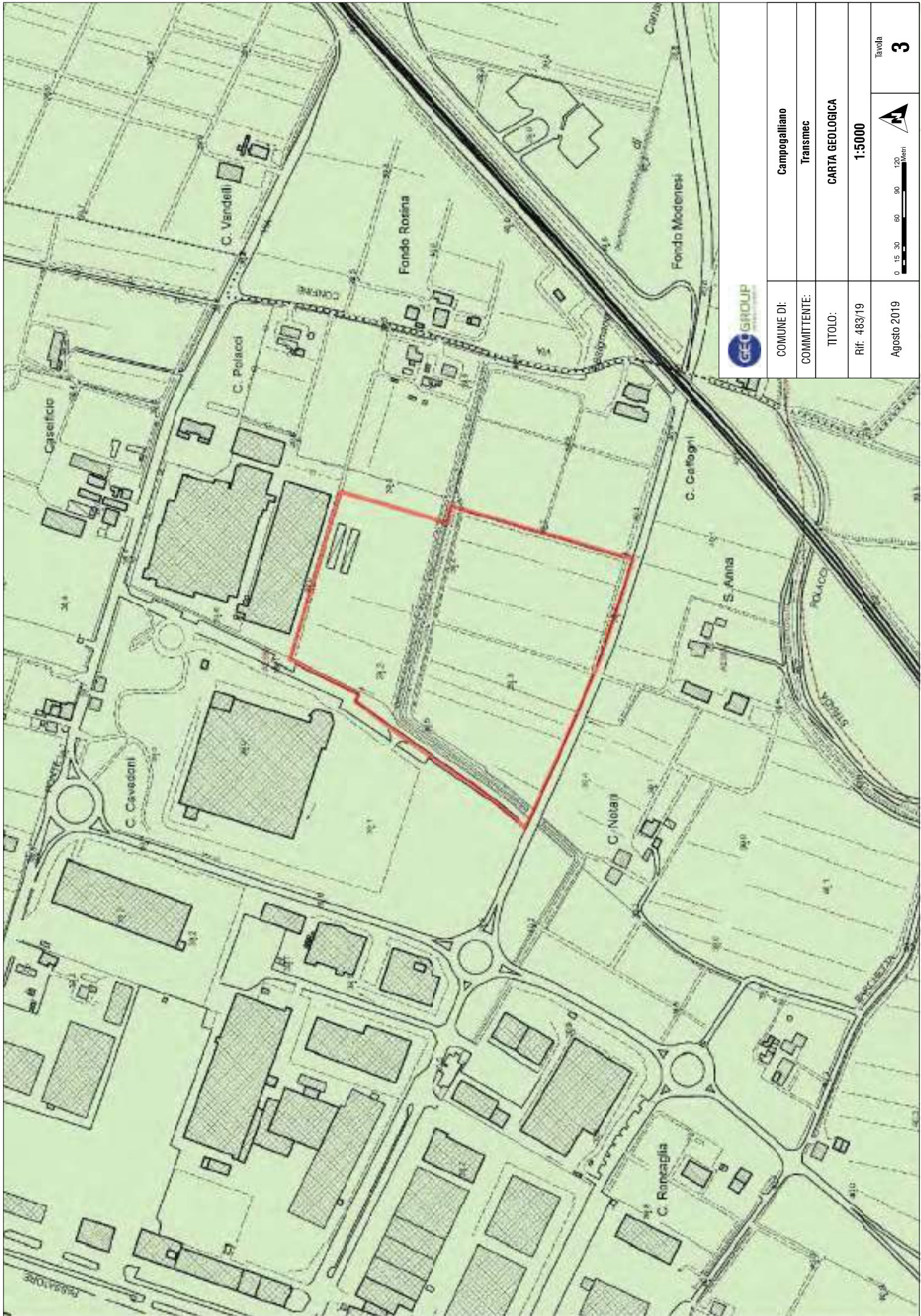
Tavole



COMUNE DI:	Campogalliano
COMMITTENTE:	Transmec
TITOLO:	CARTA TOPOGRAFICA
Rif:	489/19
Agosto 2019	1:3000
	0 5 10 20 30 40 meters
	1 tavola



COMUNE DI:	Campogalliano
COMMITTENTE:	Transmec
TITOLO:	RIPRESA FOTOGRAFICA AEREA
Rif:	483/19
Agosto 2019	
	1:3000
	
	
	Tavola
	2



COMUNE DI:	Campogalliano
COMMITTENTE:	Transnec
TITOLO:	CARTA GEOLOGICA
Rif:	483/19
Agosto 2019	1:5000



GEO GROUP s.r.l.

Indagini geognostiche, geofisiche e consulenze geologiche e geotecniche
182, via C. Costa 41100 Modena - Tel. 059/3967169 - Fax. 059/5960176 - E-mail: info@geogroupmodena.it




Tav. n. 4 "Ubicazione indagini geognostiche" (tratta da Google Maps) Scala grafica


LEGENDA

Campagna di indagini precedente

-  Prove penetrometriche statiche CPT
-  Prove penetrometriche statiche CPTU
-  Indagini sismiche HVSR
-  Indagini sismica MASW
-  Indagini sismica REMI

Campagna di indagini integrative CPTU

-  Prove penetrometriche statiche CPTU eseguite in data 11-06-2020

 Impronta di progetto dei n. 2 capannoni



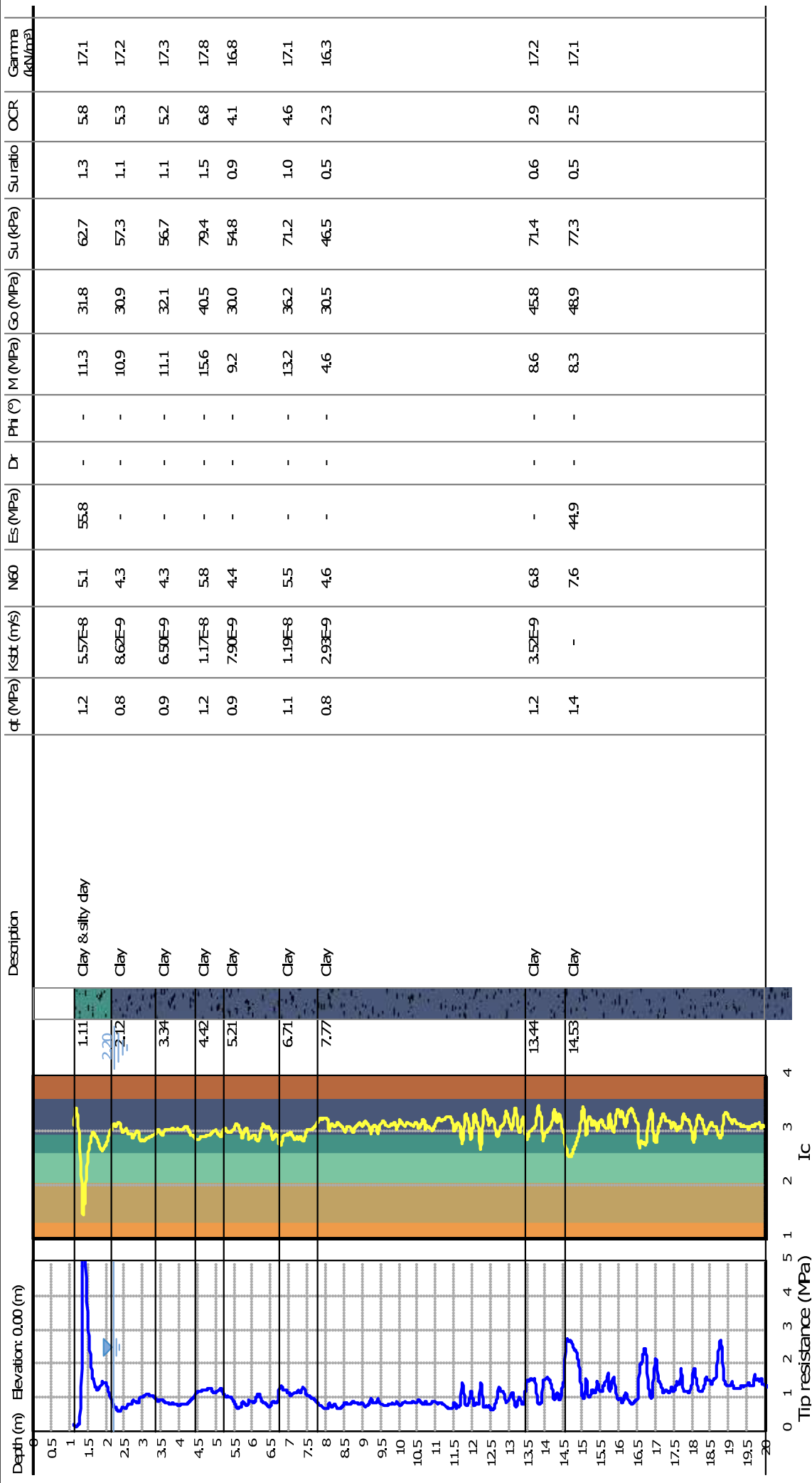
ALL.1

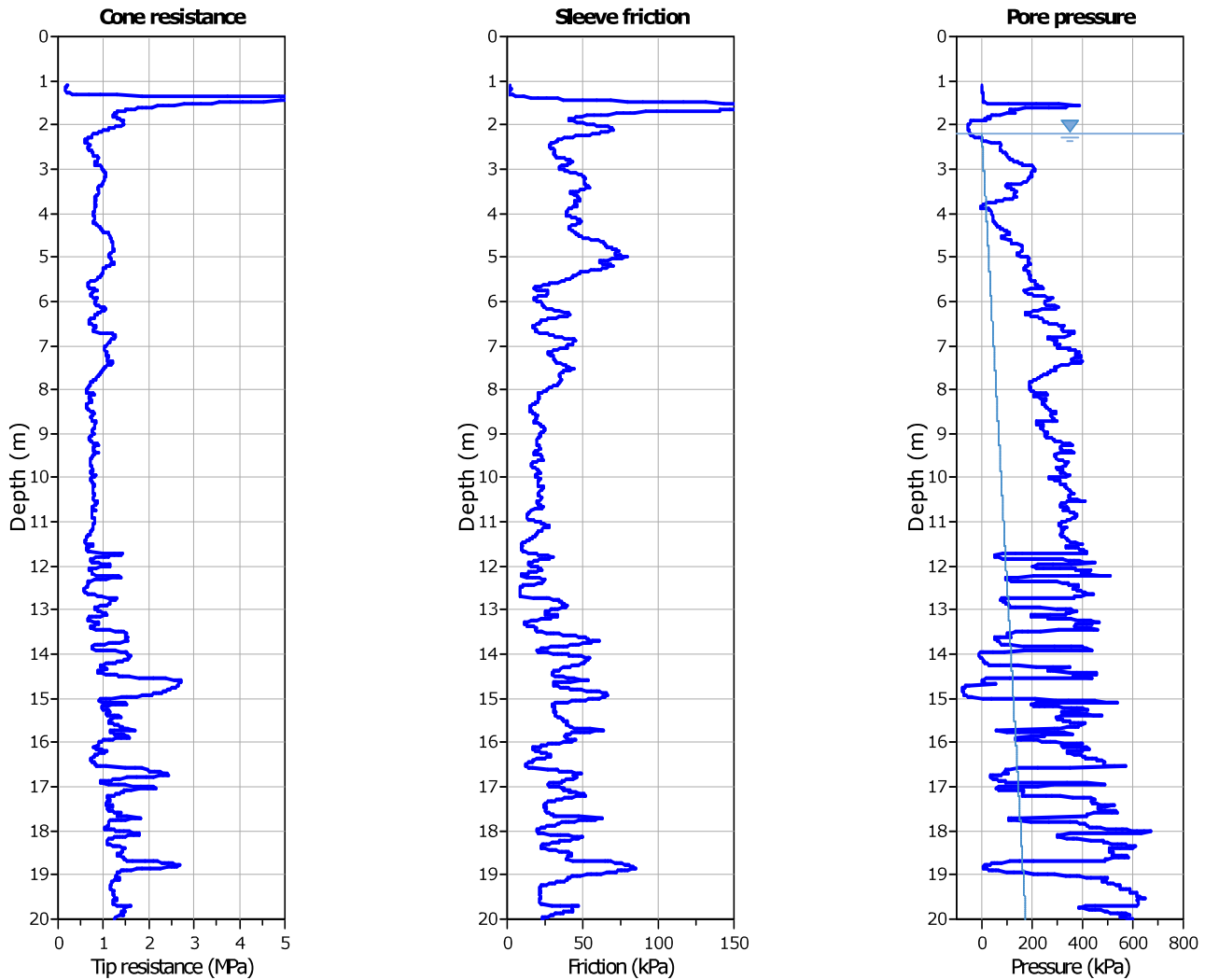
Prove penetrometriche statiche con punta elettrica e piezocono CPTu



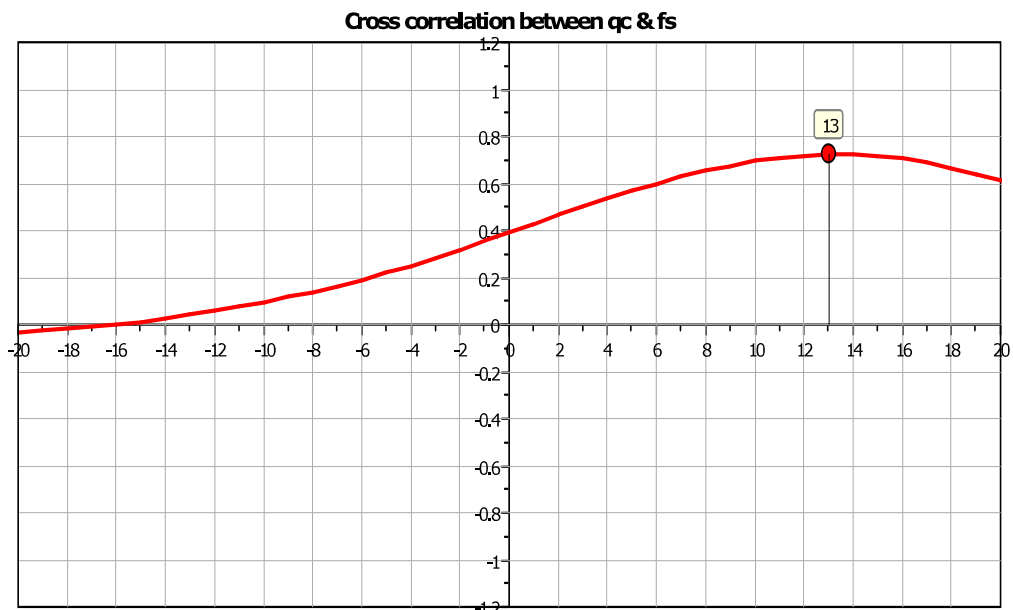
UFFICI: via Per Modena, 12 - 41051 Castelnuovo R. (MO) SEDE LEGALE: via C. Costa, 182 - 41123 Modena (MO)
tel. 059/3967169; 059/828367 - FAX 059/5960176 p. Iva e C.F. 02981500362
Email: info@geogroupmodena.it Pec: geo.group@winpec.it Sito: www.geogroupmodena.it
Operatore APR certificato ENAC per operazioni non critiche – Rif. ENAC 18627

CPTu 1

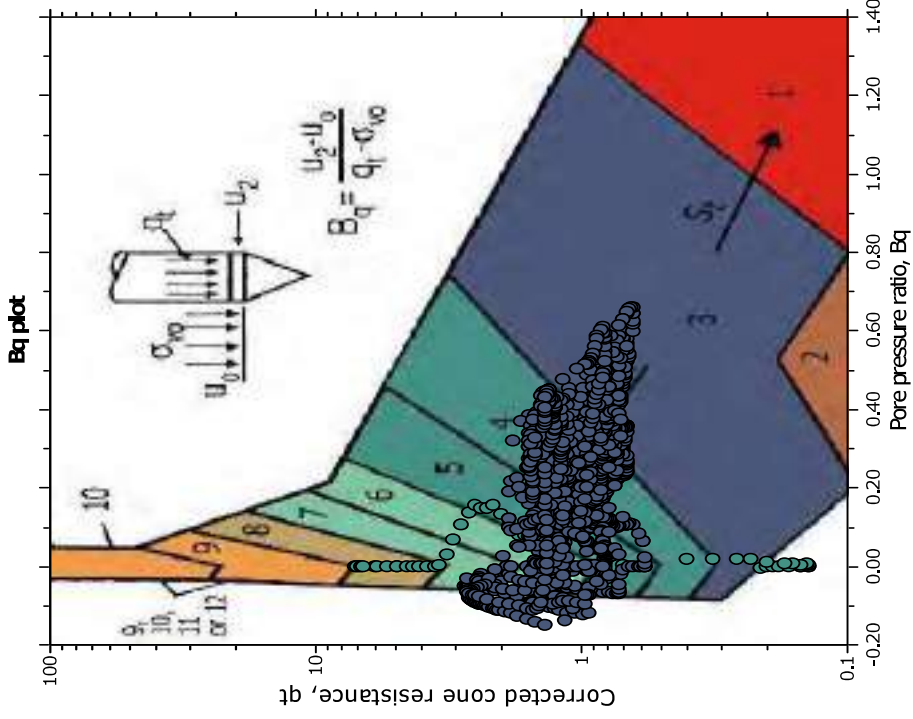
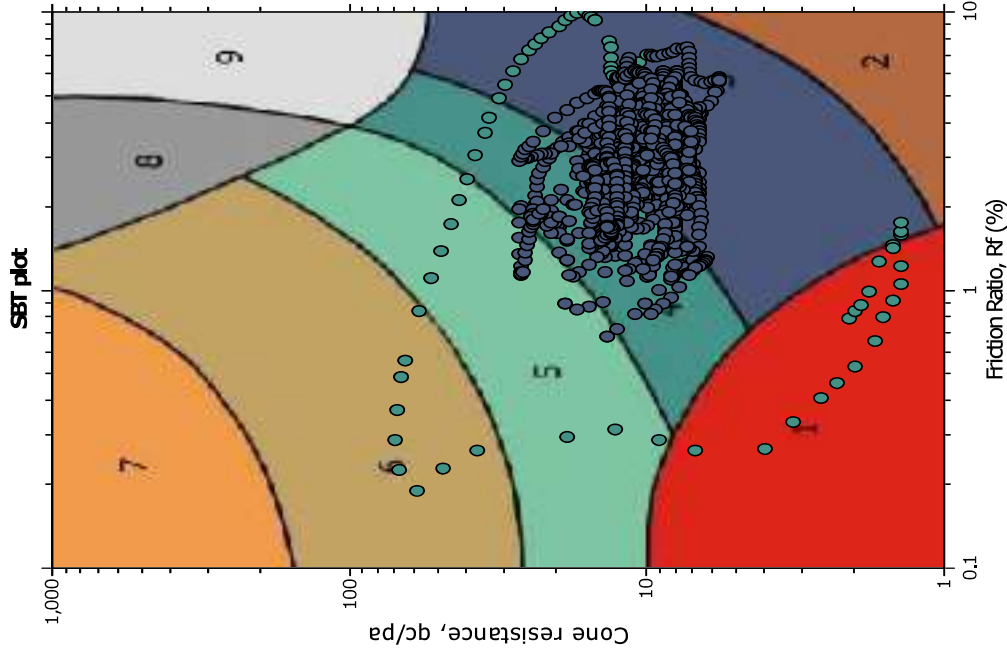




The plot below presents the cross correlation coefficient between the raw qc and fs values (as measured on the field). X axes presents the lag distance (one lag is the distance between two successive CPT measurements).



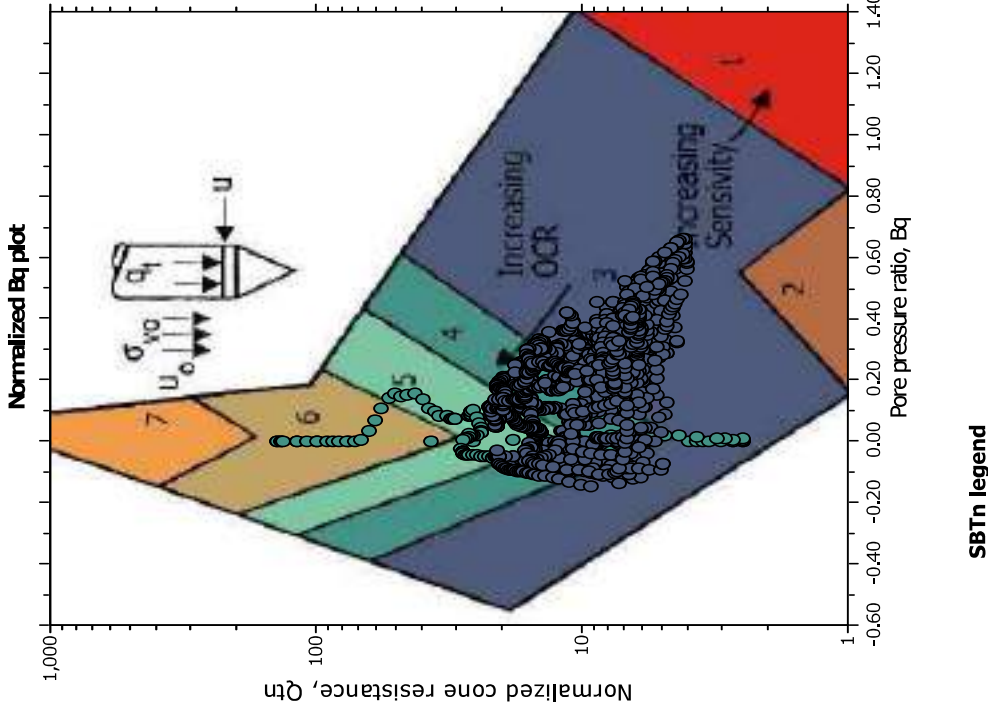
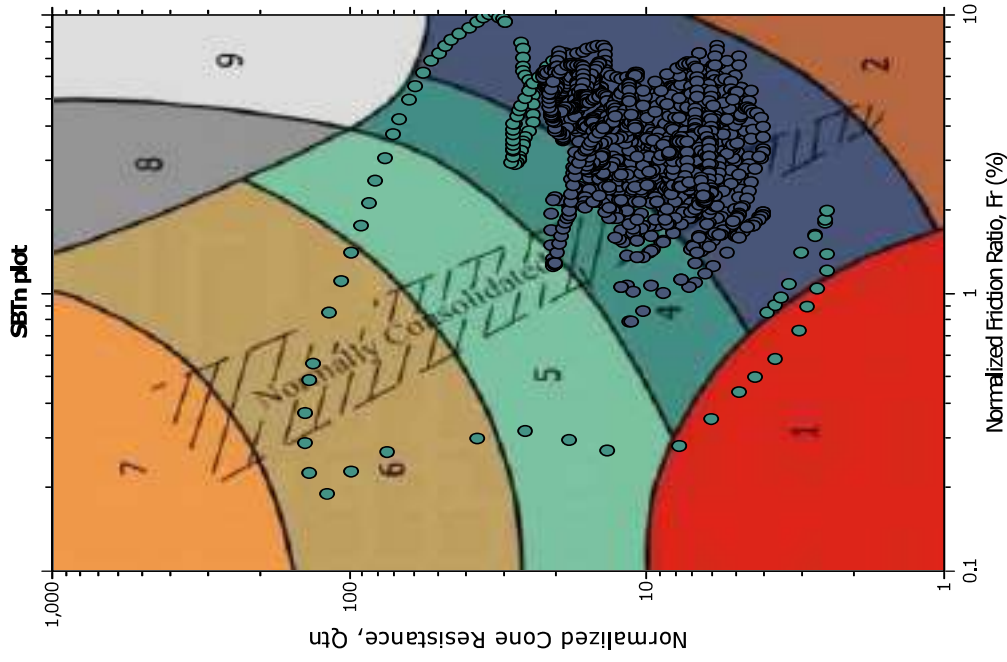
SBT - Bq plots

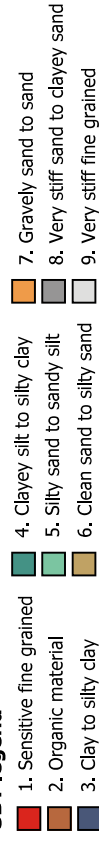
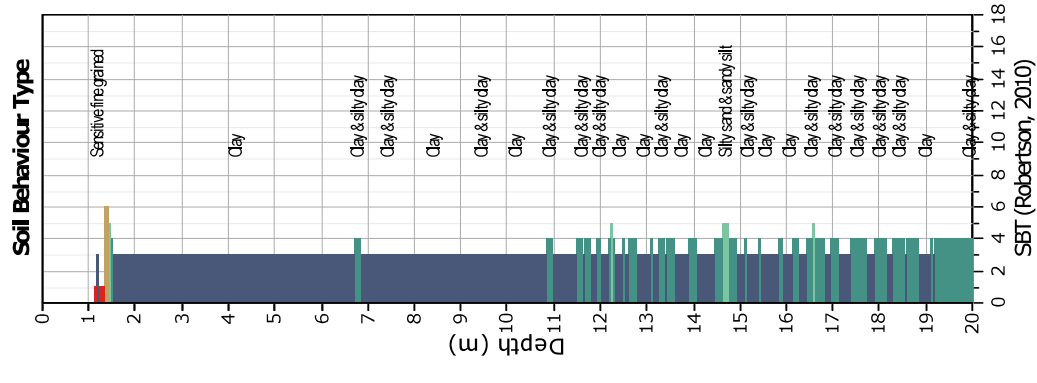
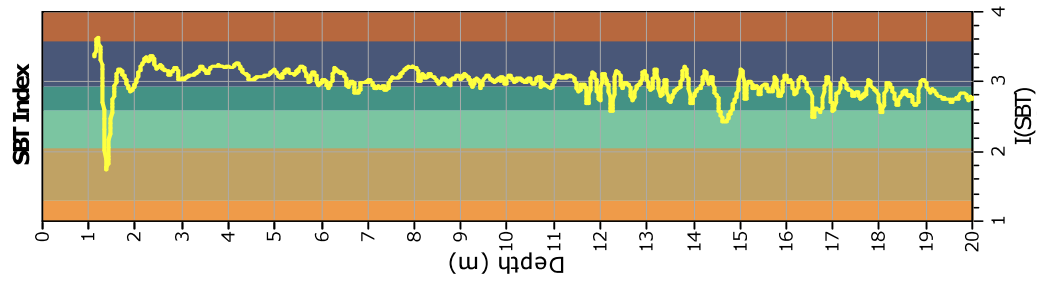
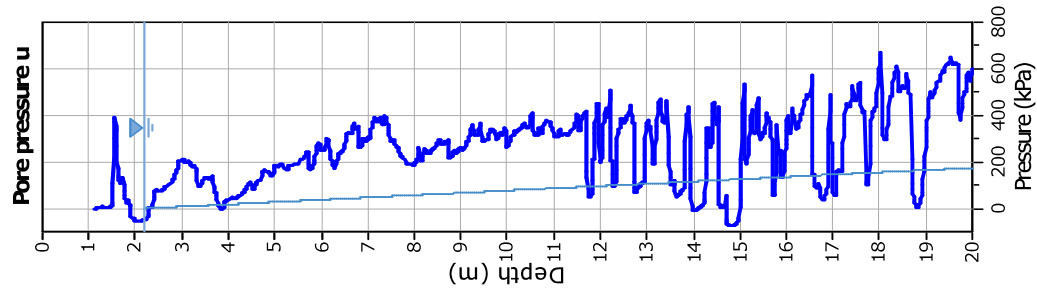
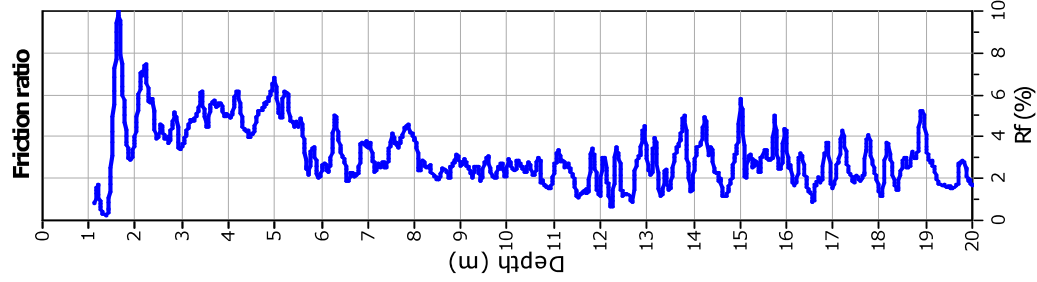
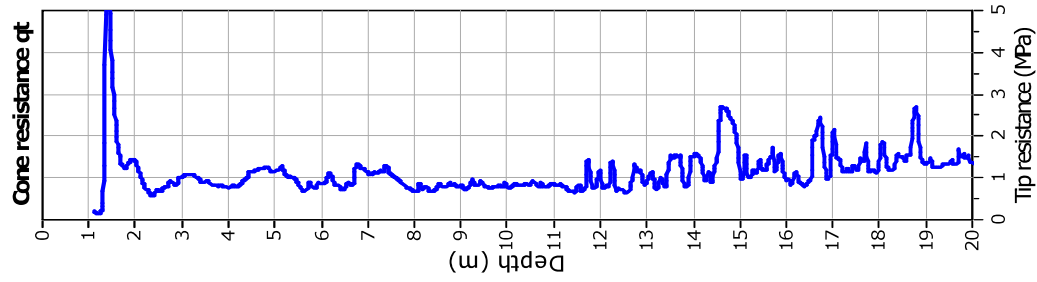


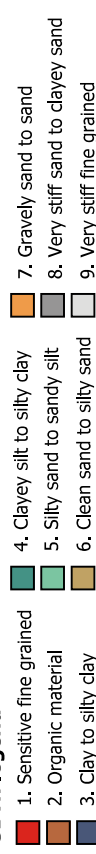
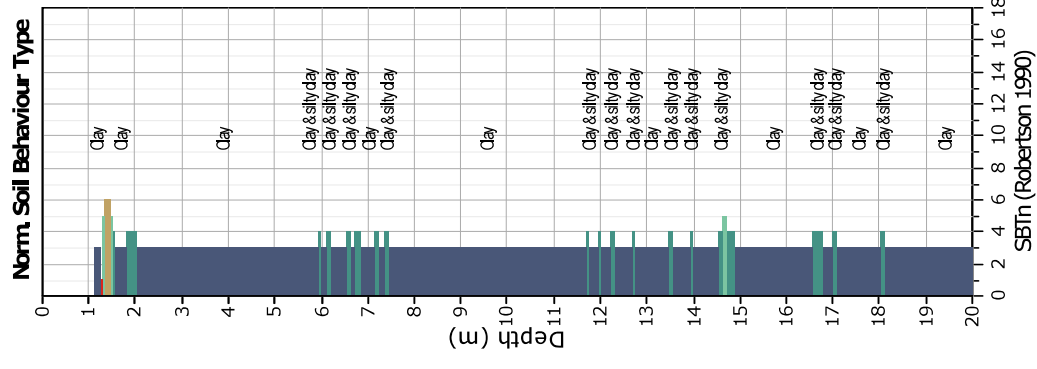
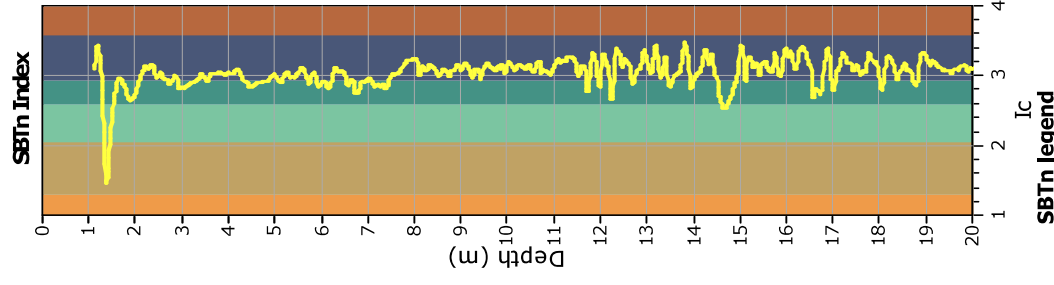
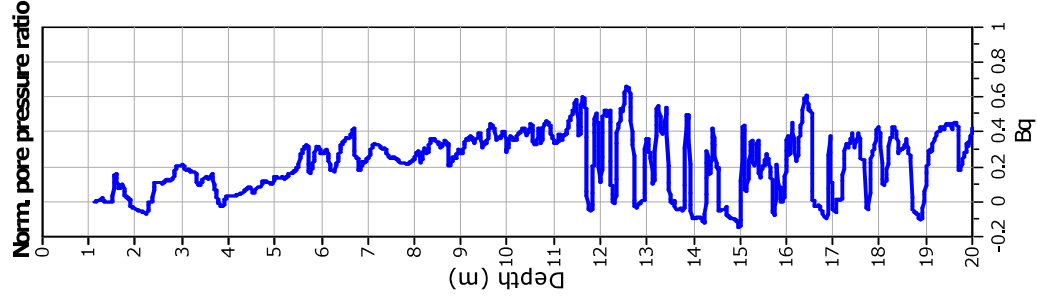
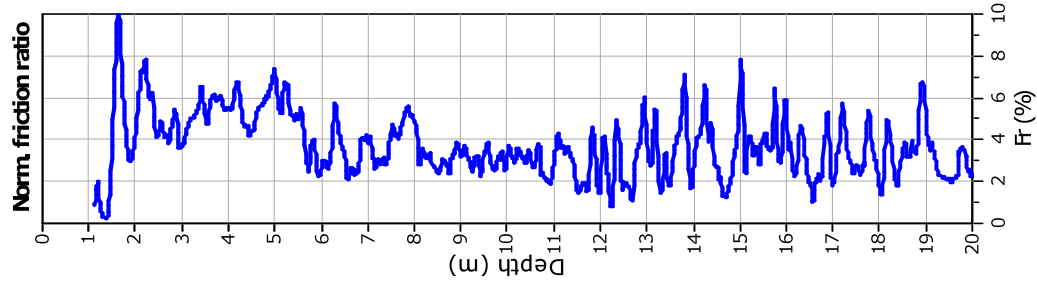
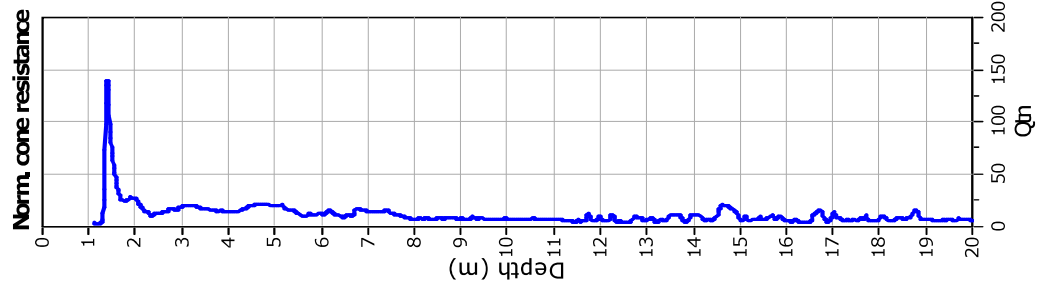
SBT legend

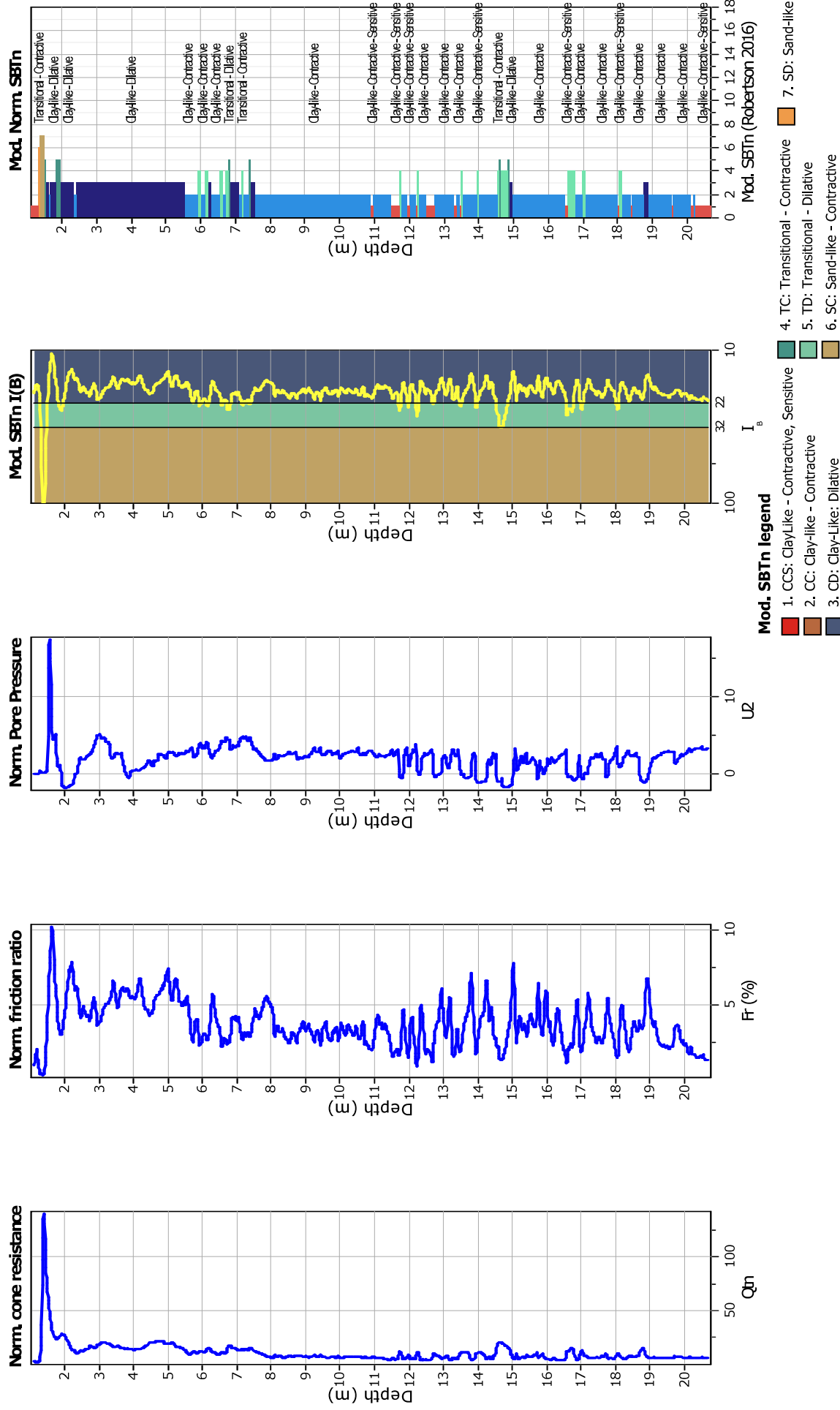
- 1. Sensitive fine grained
- 2. Organic material
- 3. Clay to silty clay
- 4. Clayey silt to silty clay
- 5. Silty sand to sandy silt
- 6. Clean sand to silty sand
- 7. Gravely sand to sand
- 8. Very stiff sand to clayey sand
- 9. Very stiff fine grained

SBT - Bq plots (normalized)

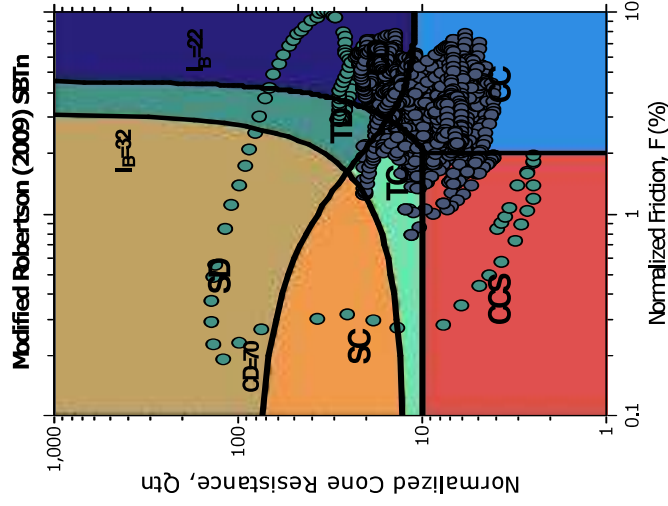




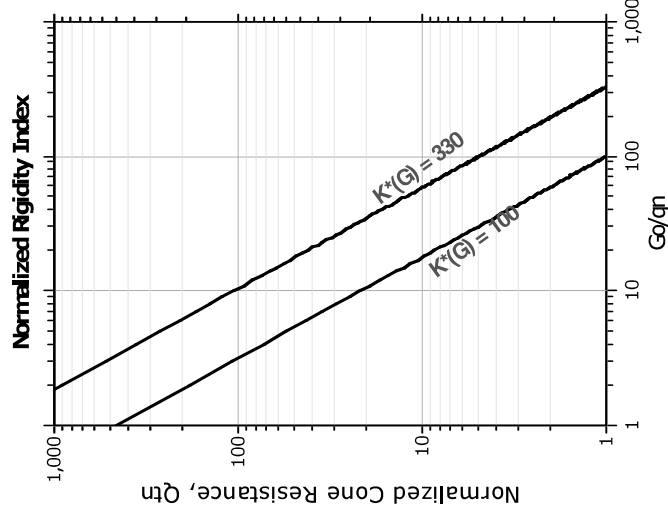
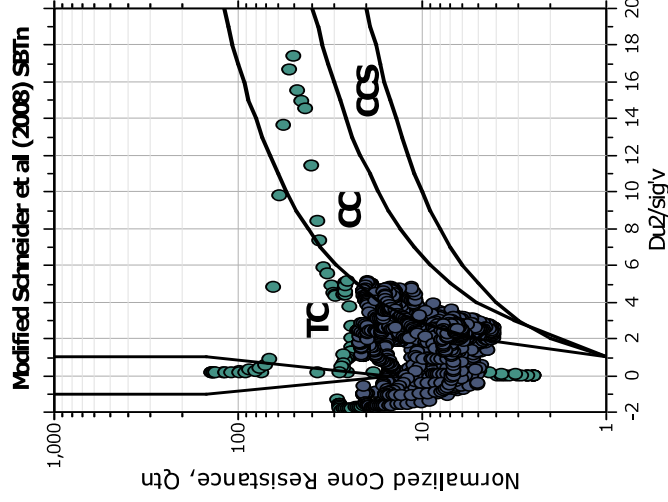




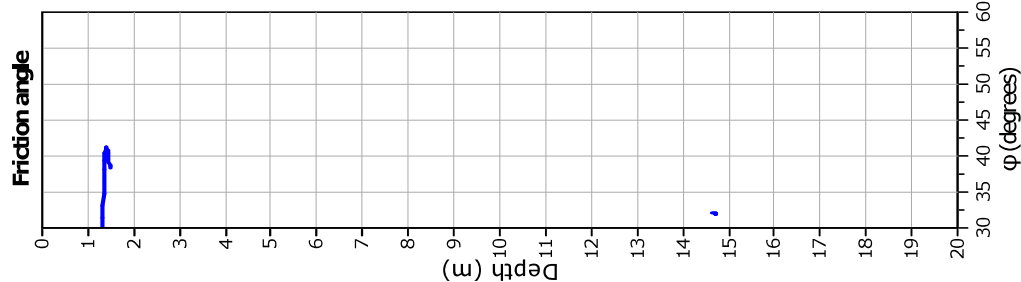
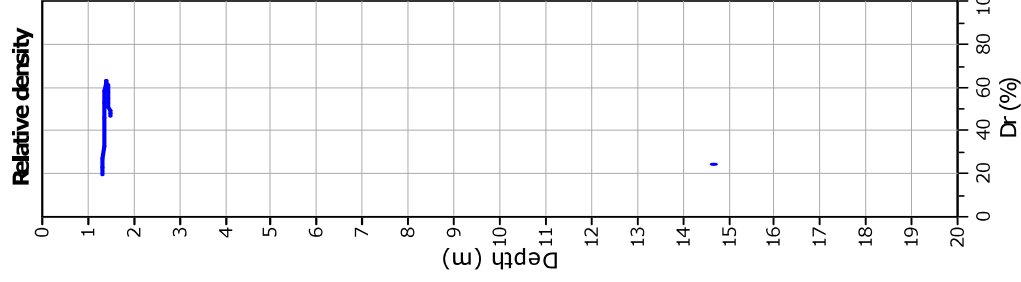
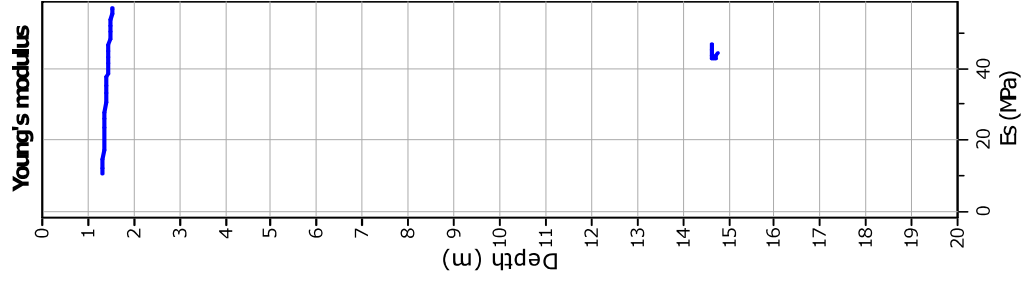
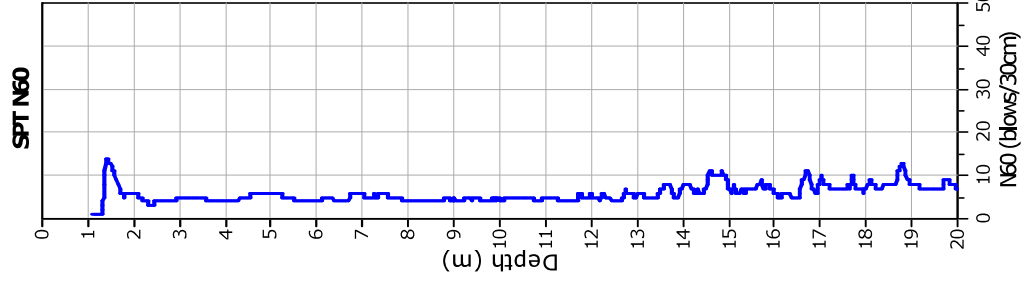
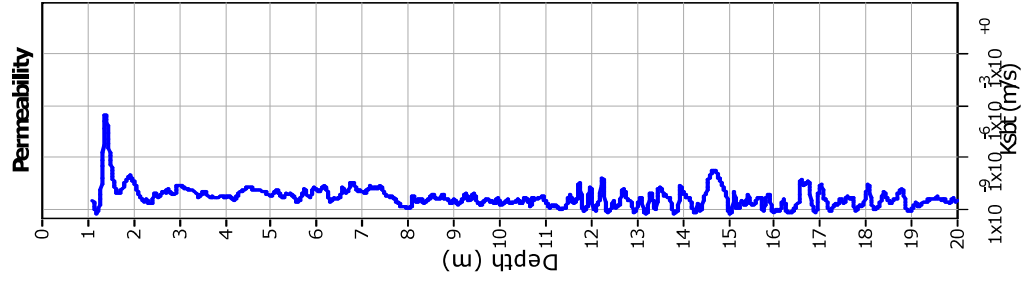
Updated SBTn plots



- CCS: Clay-like - Contractive - Sensitive
- CC: Clay-like - Contractive
- CD: Clay-like - Dilative
- TC: Transitional - Contractive
- TD: Transitional - Dilative
- SC: Sand-like - Contractive
- SD: Sand-like - Dilative

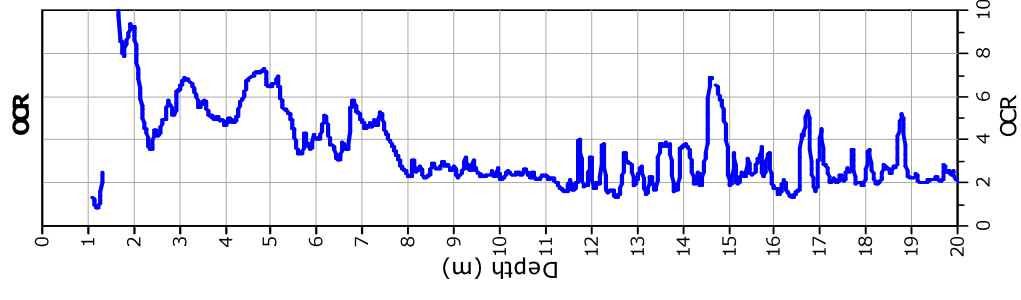
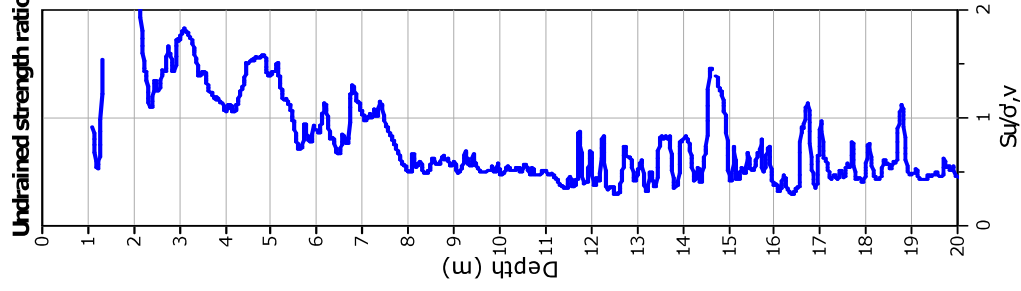
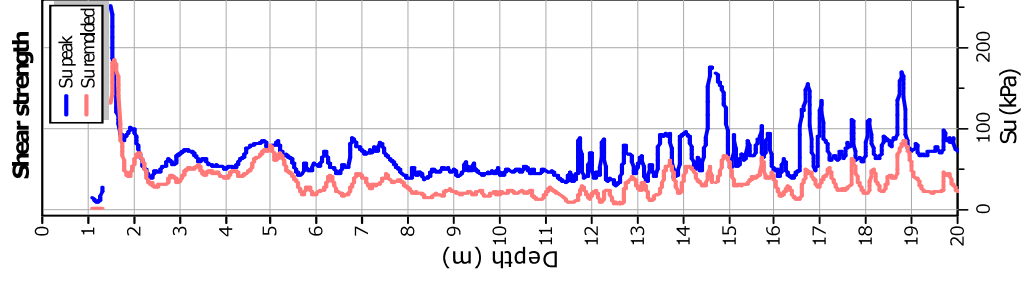
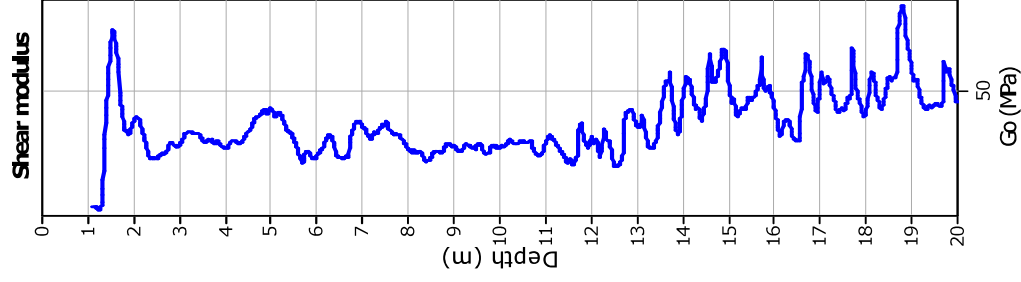
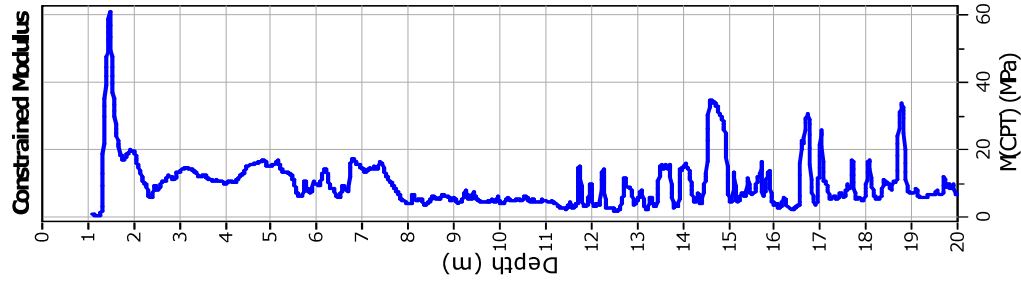


$K(G) > 330$: Soils with significant microstructure (e.g. age/cementation)



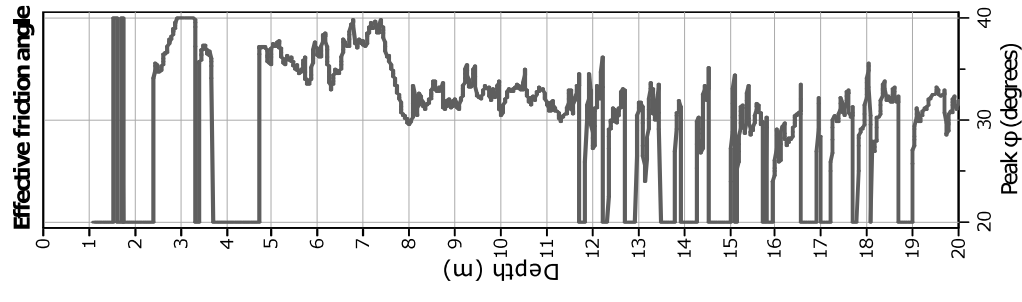
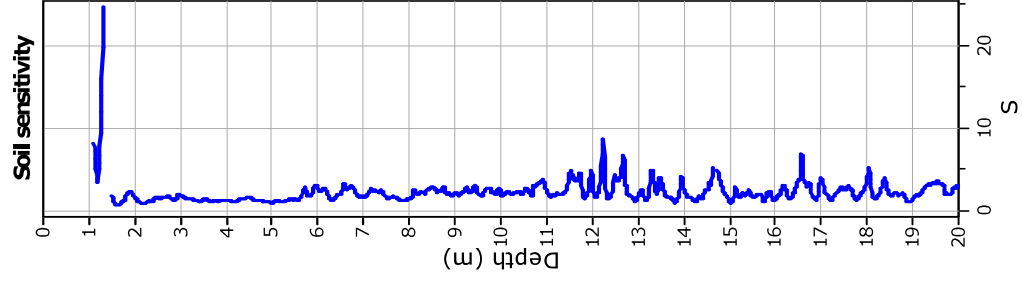
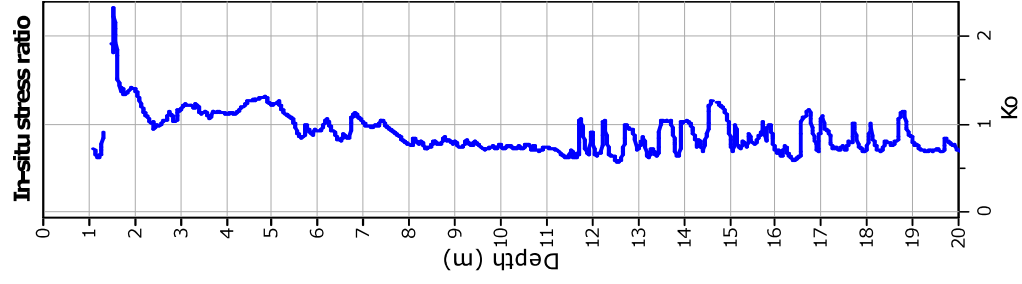
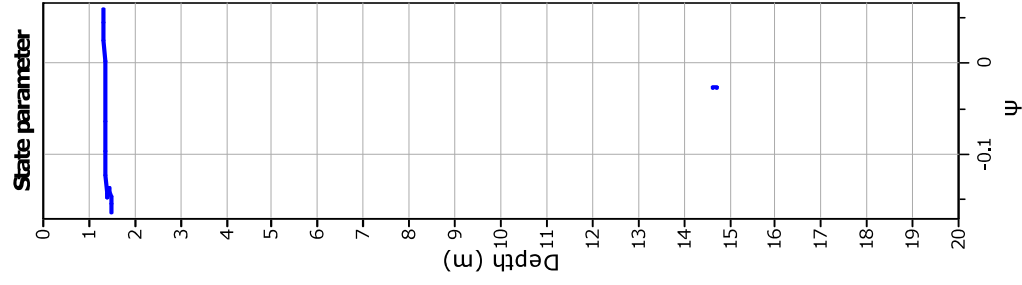
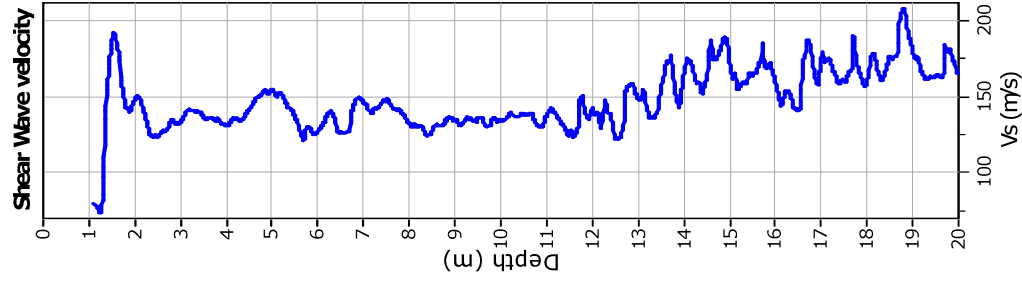
Calculation parameters

Permeability: Based on SBT_n
 SPT N_{60} : Based on I_c and q_t
 Young's modulus: Based on variable alpha using I_c (Robertson, 2009) ● — User defined estimation data
 Relative density constant, C_{r1} : 350.0
 Phi: Based on Kulhawy & Mayne (1990)



Calculation parameters

Constrained modulus: Based on variable α/β using I_c and Q_m (Robertson, 2009) OCR factor for clays, N_{kt} : 0.33
 Go: Based on variable α/β using I_c (Robertson, 2009) —●— User defined estimation data
 Undrained shear strength cone factor for clays, N_{kt} : 14 —●— Flat Dilatometer Test data



Calculation parameters

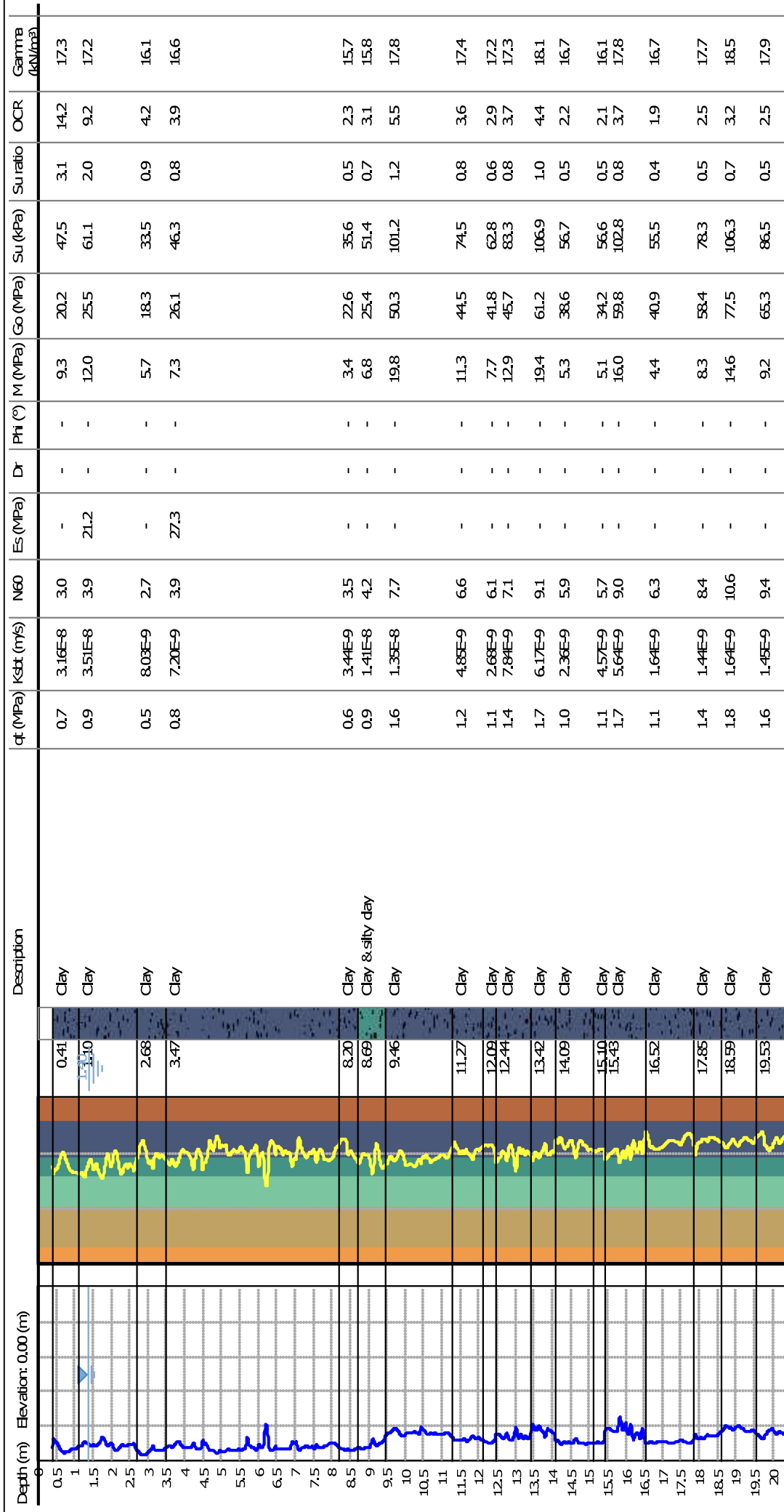
Soil Sensitivity factor, N_s : 7.00

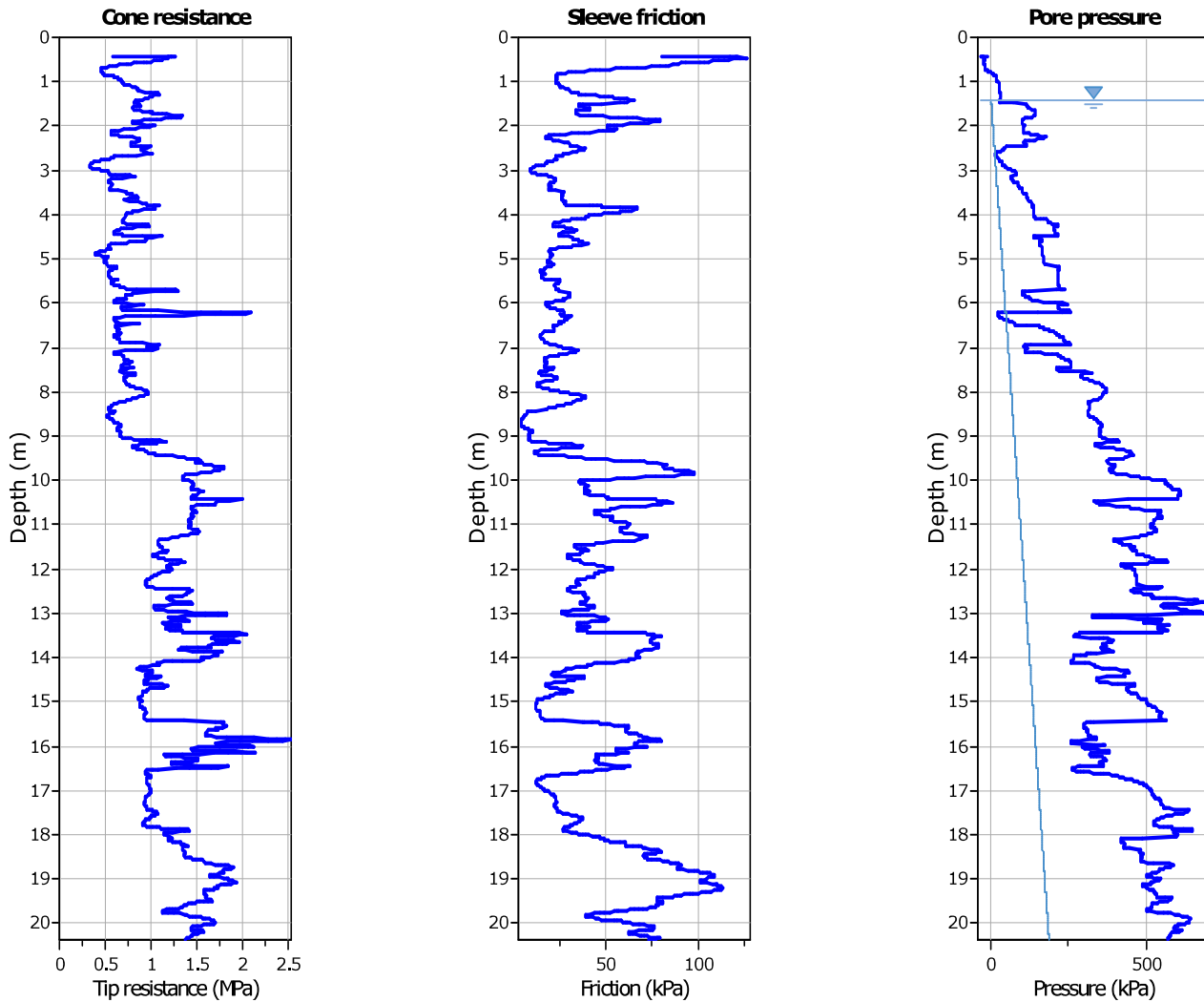
—●— User defined estimation data



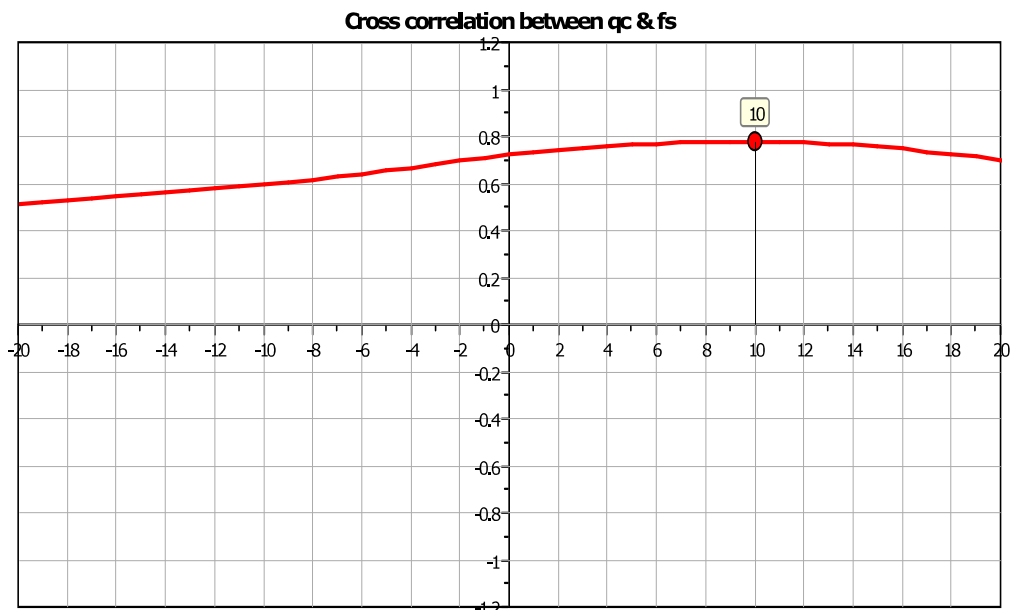
UFFICI: via Per Modena, 12 - 41051 Castelnuovo R. (MO) SEDE LEGALE: via C. Costa, 182 - 41123 Modena (MO)
tel. 059/3967169; 059/828367 - FAX 059/5960176 p. Iva e C.F. 02981500362
Email: info@geogroupmodena.it Pec: geo.group@winpec.it Sito: www.geogroupmodena.it
Operatore APR certificato ENAC per operazioni non critiche – Rif. ENAC 18627

CPTu 2

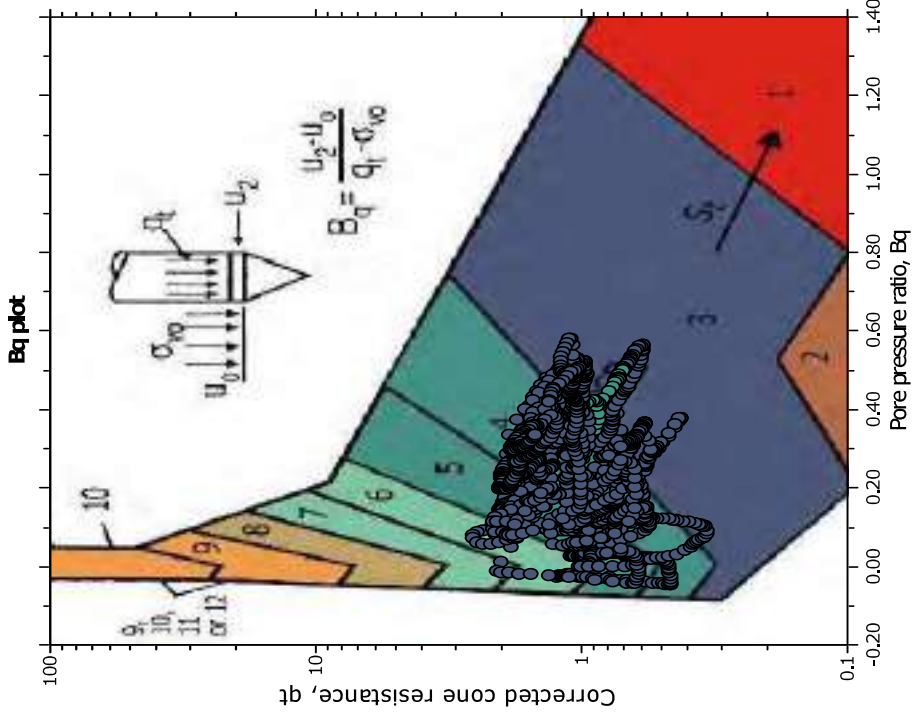
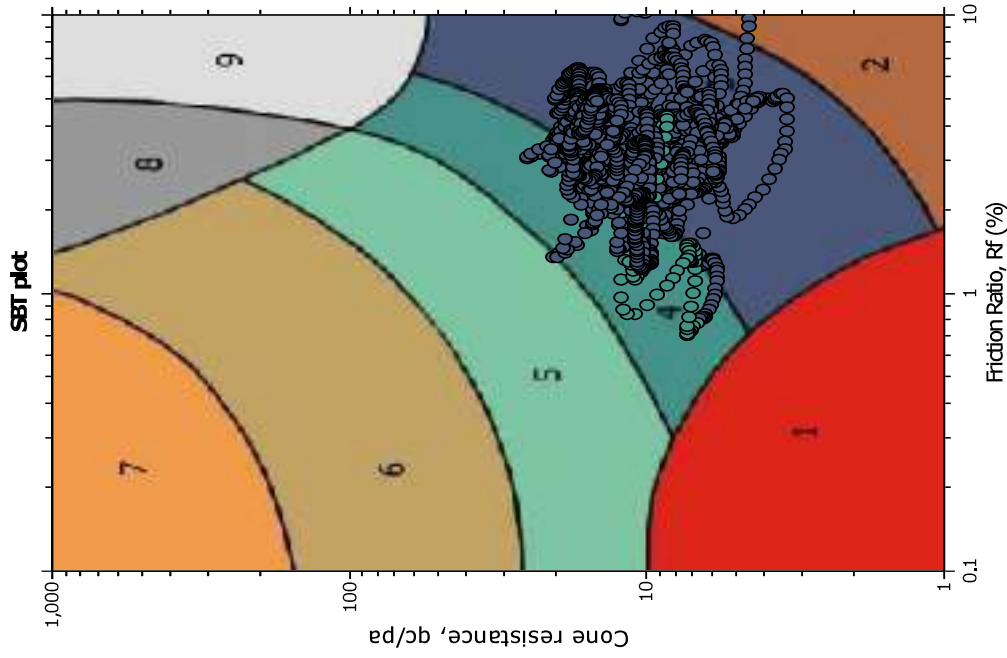




The plot below presents the cross correlation coefficient between the raw q_c and f_s values (as measured on the field). X axes presents the lag distance (one lag is the distance between two successive CPT measurements).



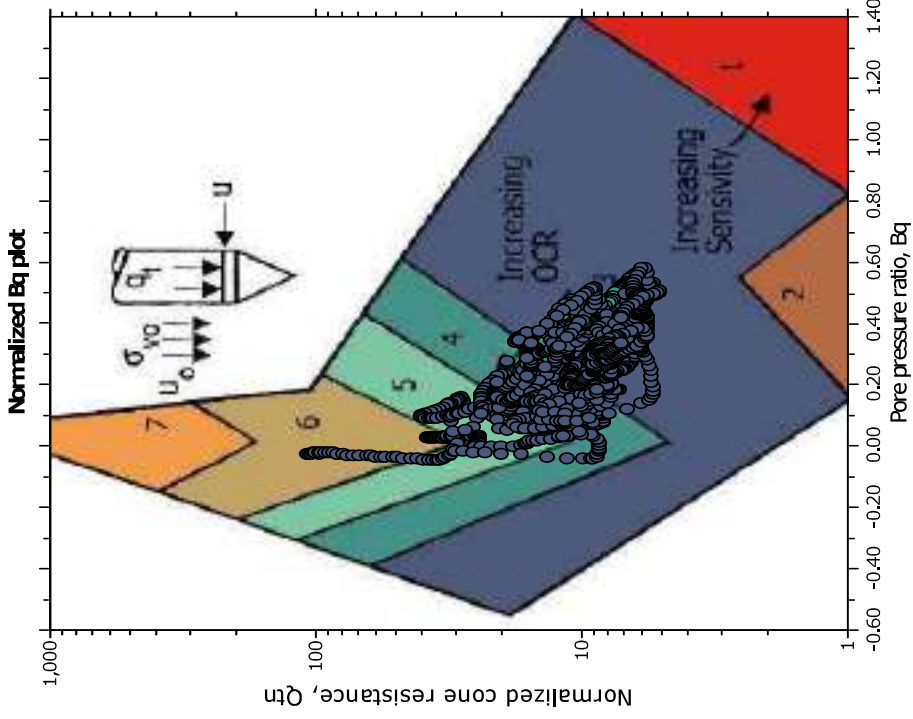
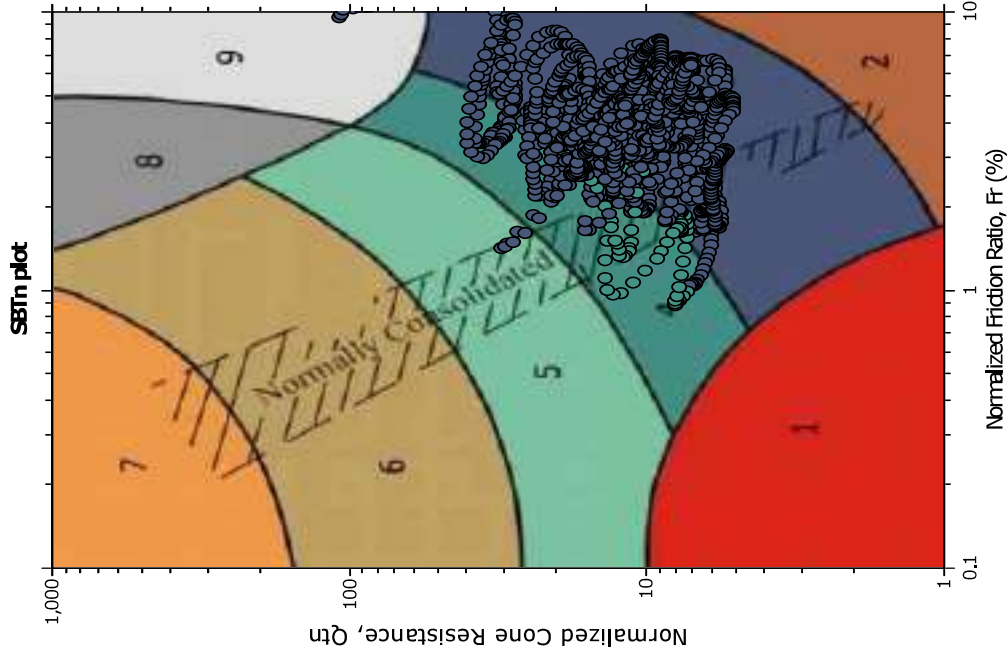
SBT - Bq plots

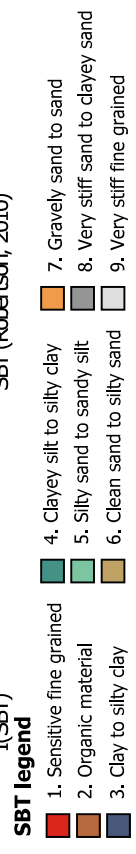
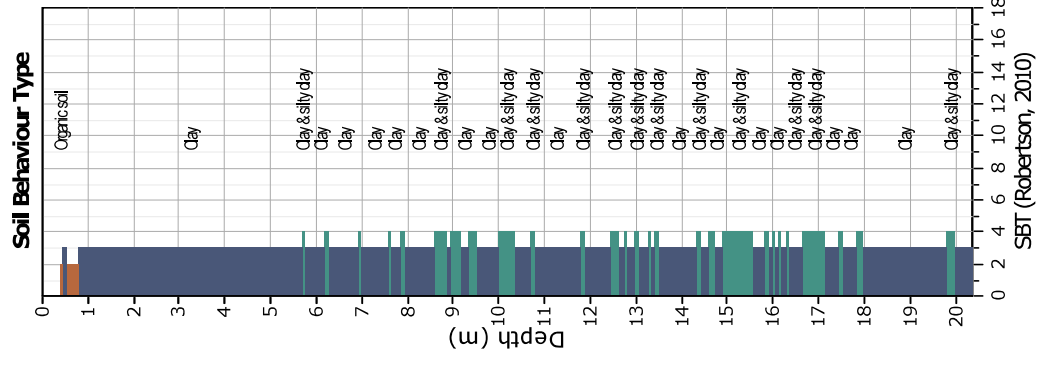
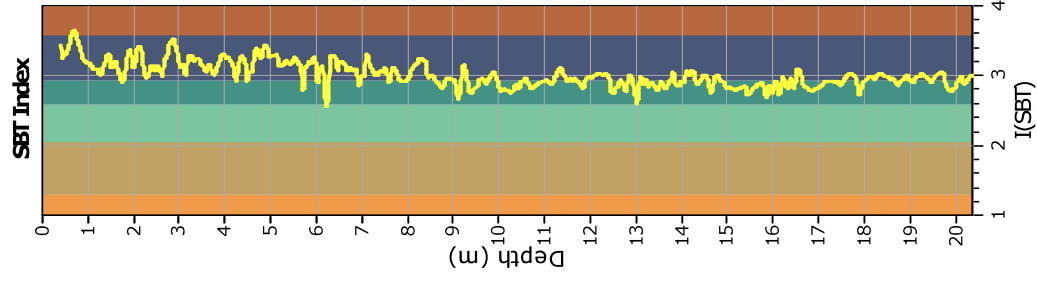
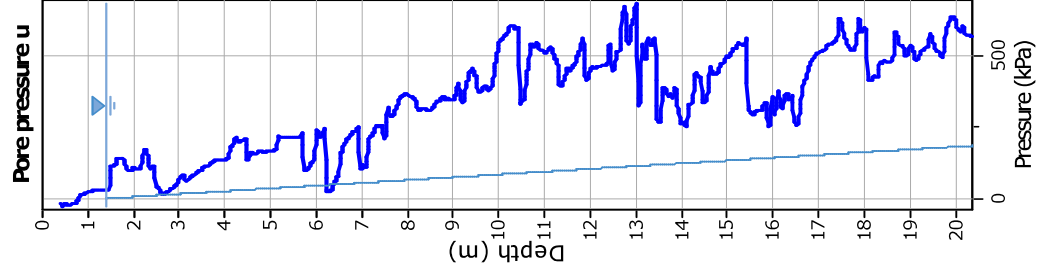
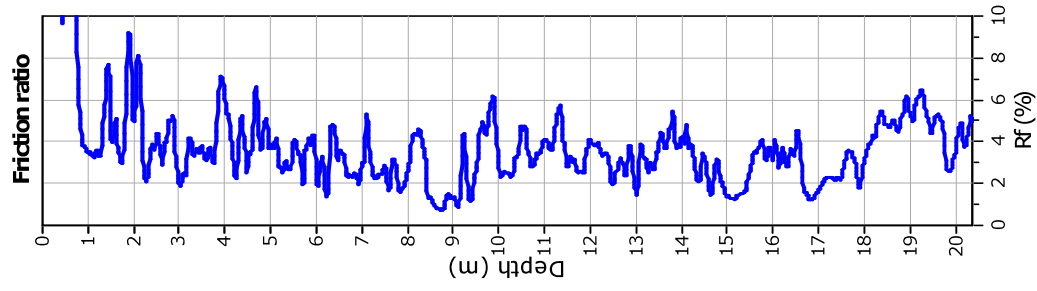
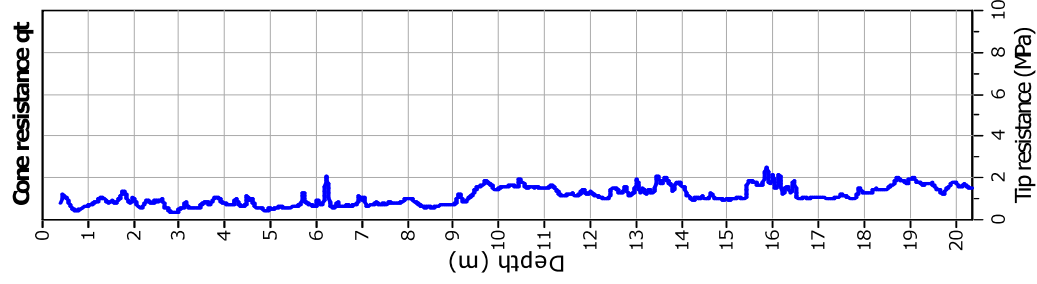


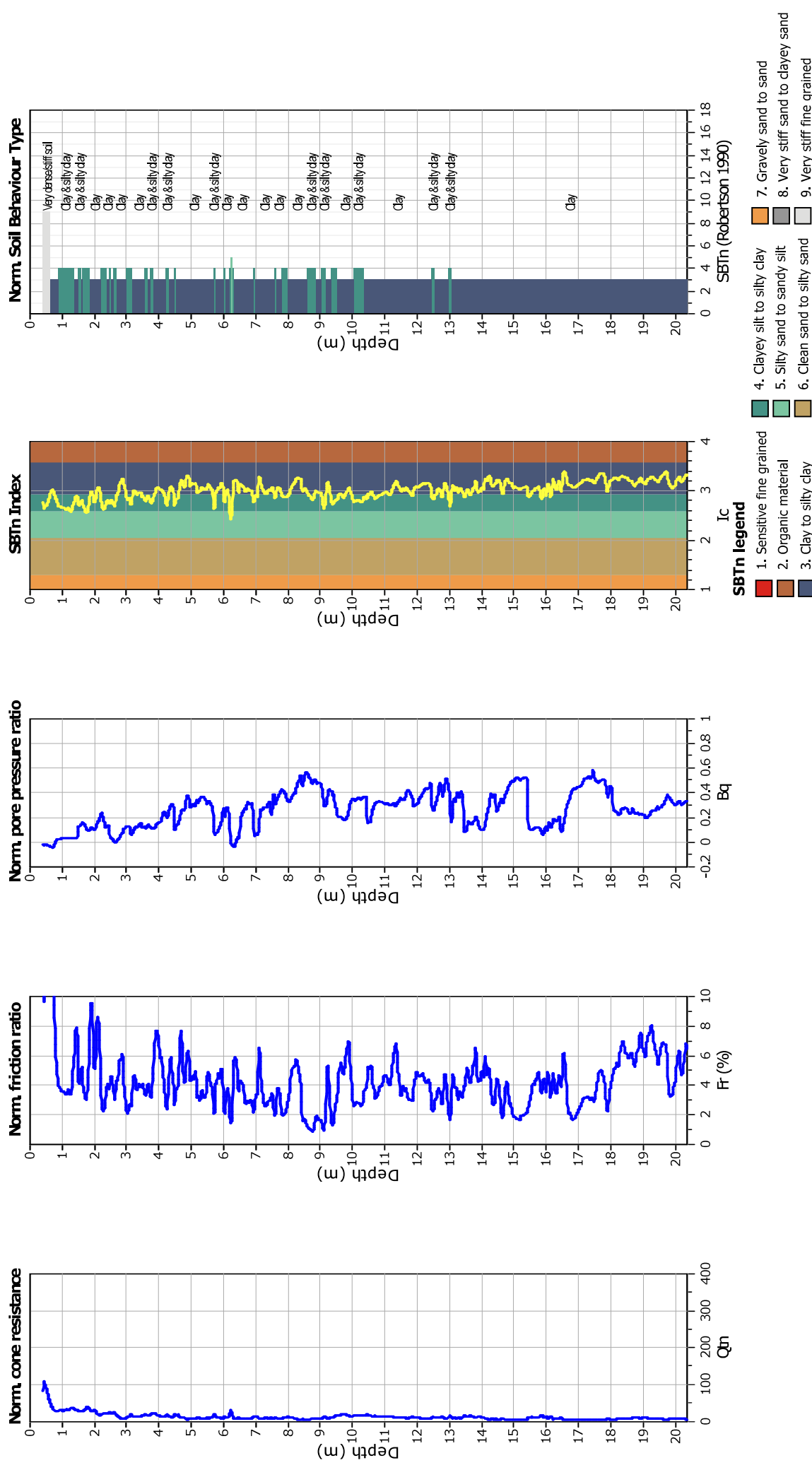
SBT legend

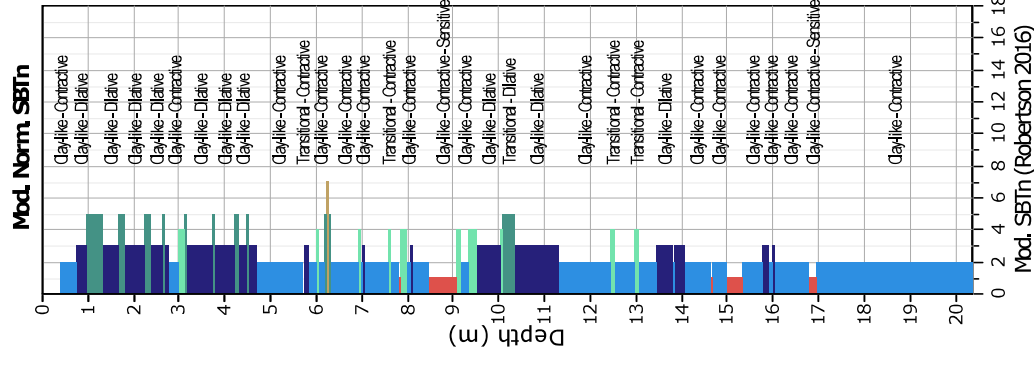
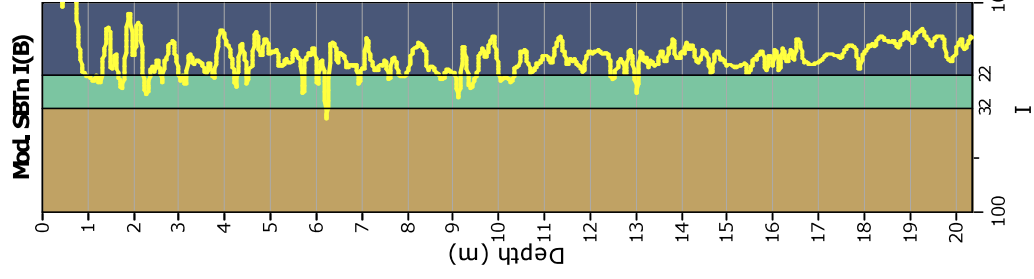
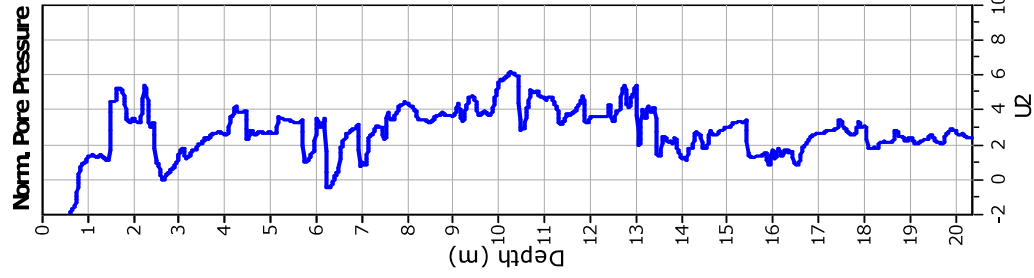
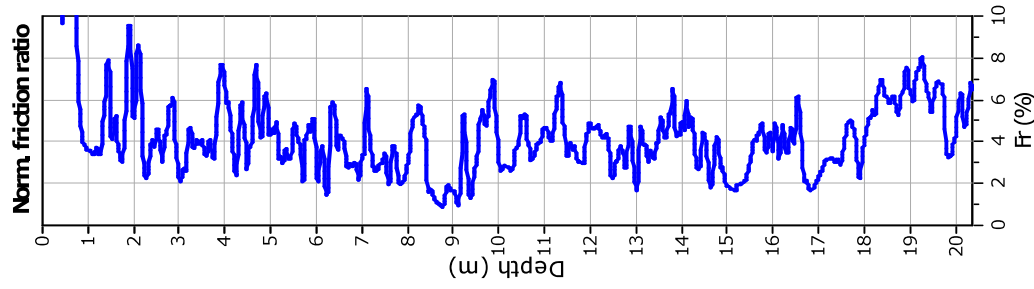
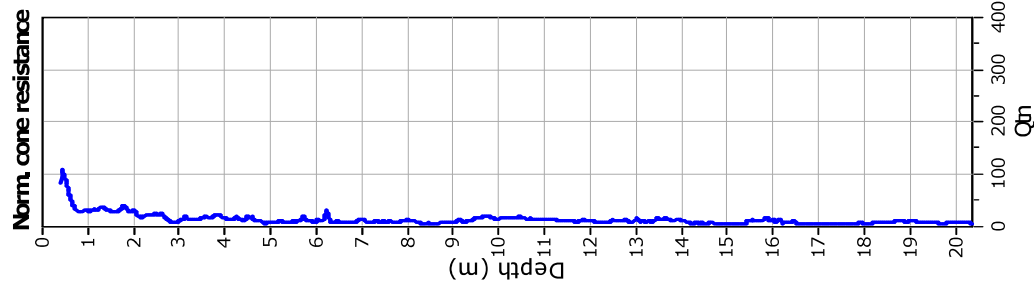
- 1. Sensitive fine grained
- 2. Organic material
- 3. Clay to silty clay
- 4. Clayey silt to silty clay
- 5. Silty sand to sandy silt
- 6. Clean sand to silty sand
- 7. Gravely sand to sand
- 8. Very stiff sand to clayey sand
- 9. Very stiff fine grained

SBT - Bq plots (normalized)





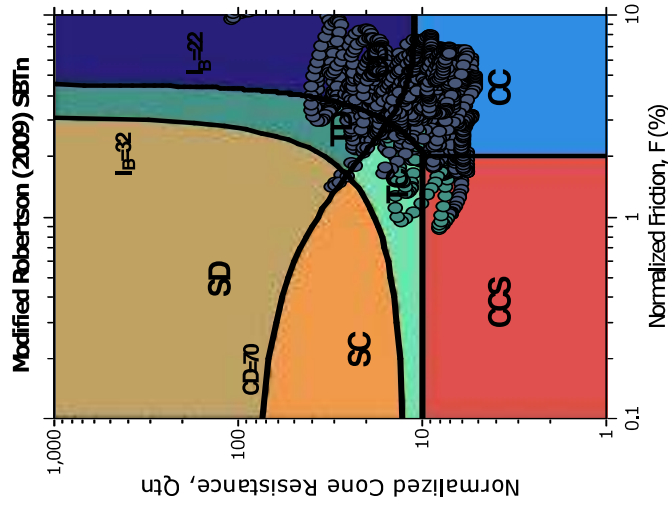




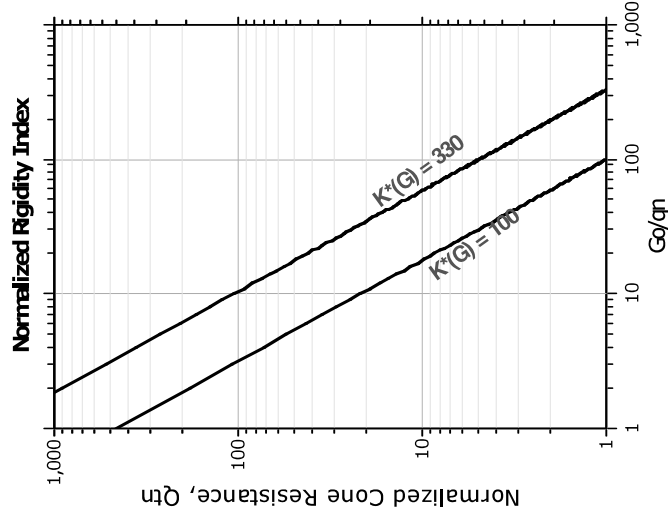
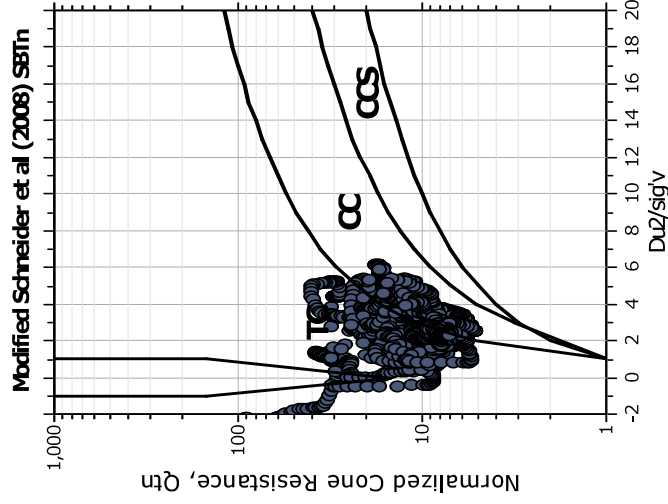
Mod. SBTn legend

- 1. CCS: ClayLike - Contractive, Sensitive
- 2. CC: Clay-like - Contractive
- 3. CD: Clay-Like: Dilative
- 4. TC: Transitional - Contractive
- 5. TD: Transitional - Dilative
- 6. SC: Sand-like - Contractive
- 7. SD: Sand-like - Dilative

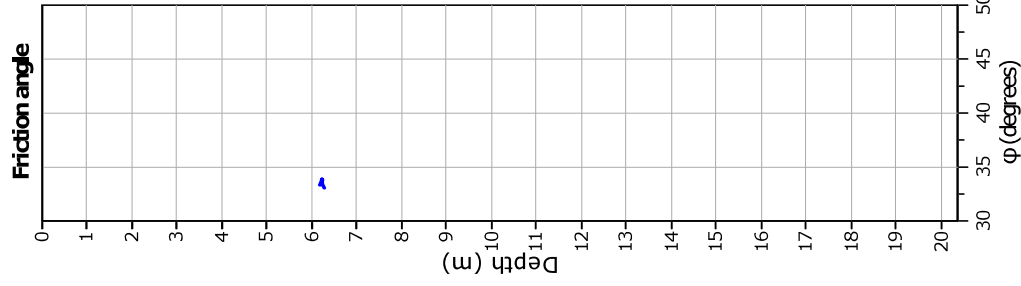
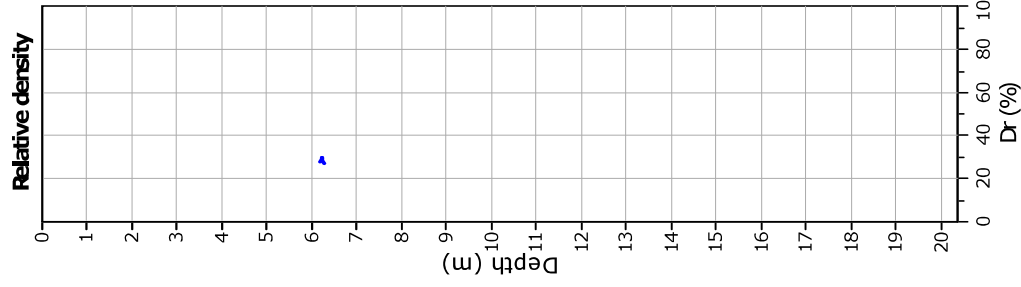
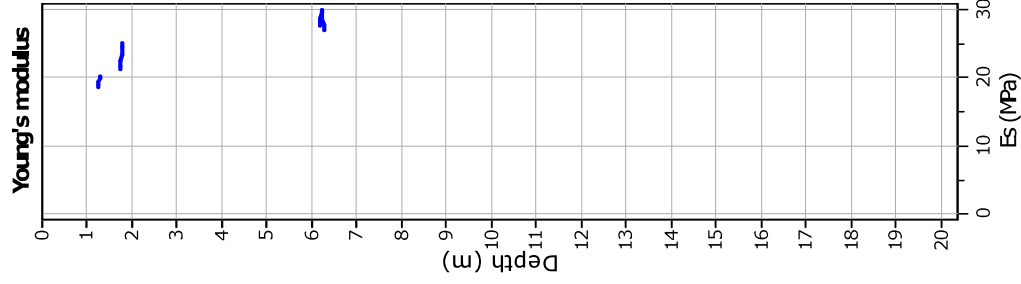
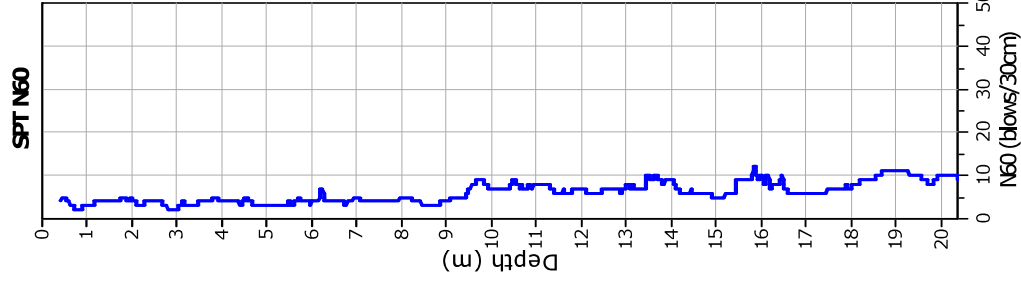
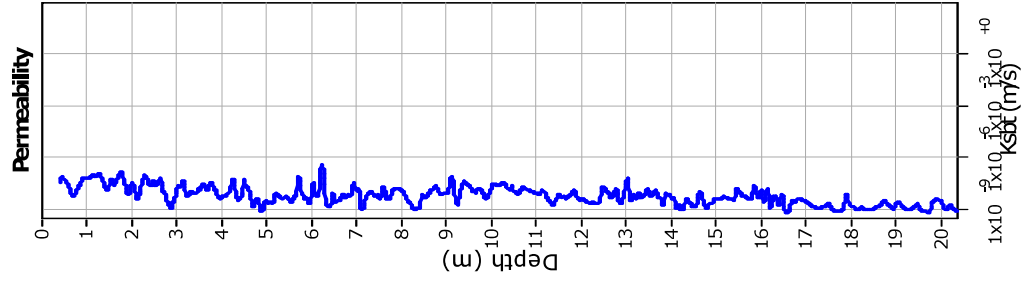
Updated SBTn plots



- CCS: Clay-like - Contractive - Sensitive
- CC: Clay-like - Contractive
- CD: Clay-like - Dilative
- TC: Transitional - Contractive
- TD: Transitional - Dilative
- SC: Sand-like - Contractive
- SD: Sand-like - Dilative



K(G) > 330: Soils with significant microstructure (e.g. age/cementation)



Calculation parameters

Permeability: Based on SBT_n

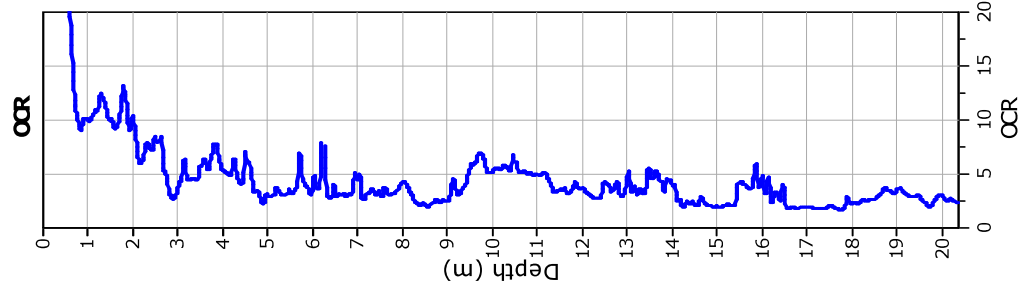
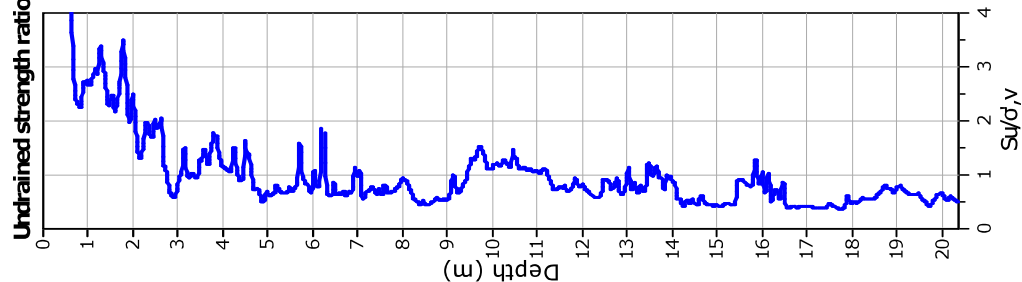
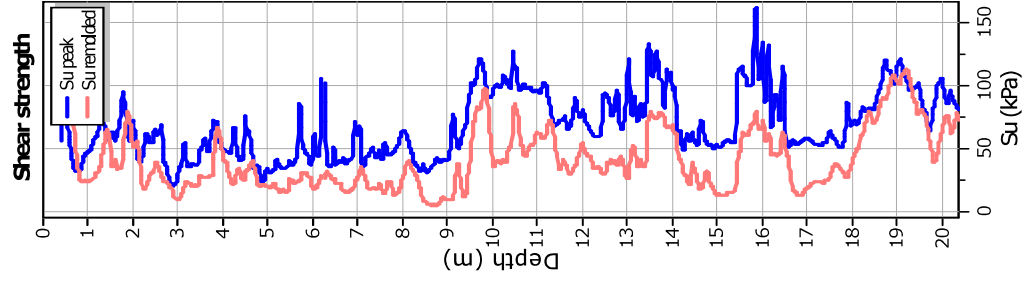
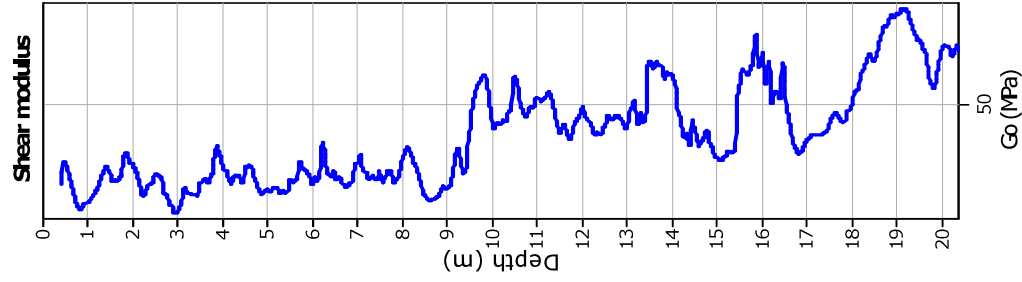
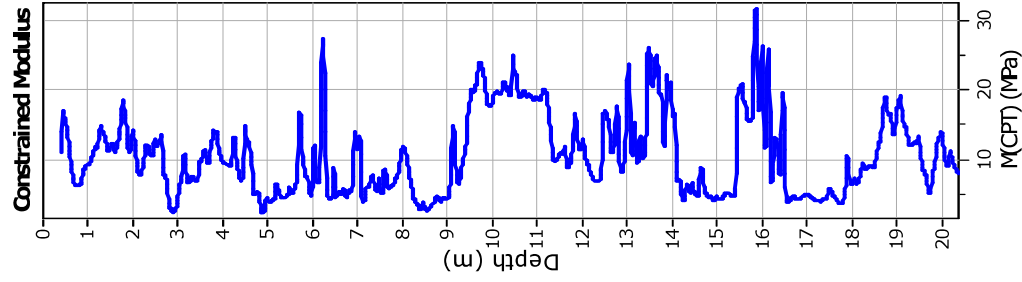
SPT N_{60} : Based on I_c and q_t

Young's modulus: Based on variable alpha using I_c (Robertson, 2009)

Relative density constant, C_{Dr} : 350.0

Phi: Based on Kulhawy & Mayne (1990)

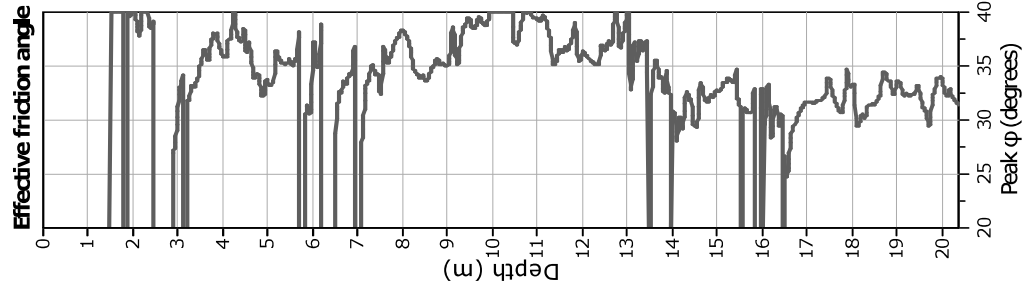
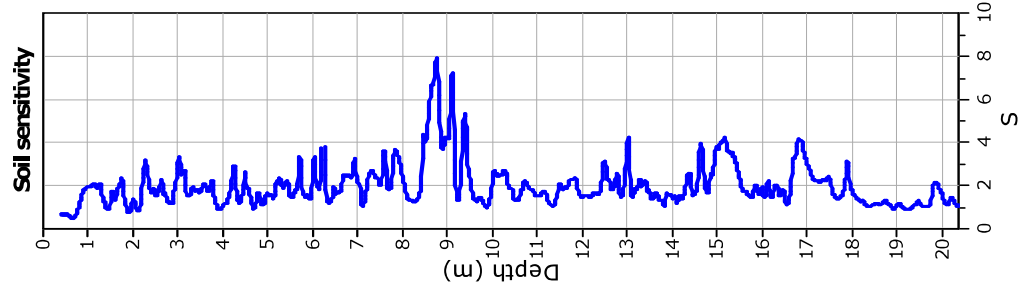
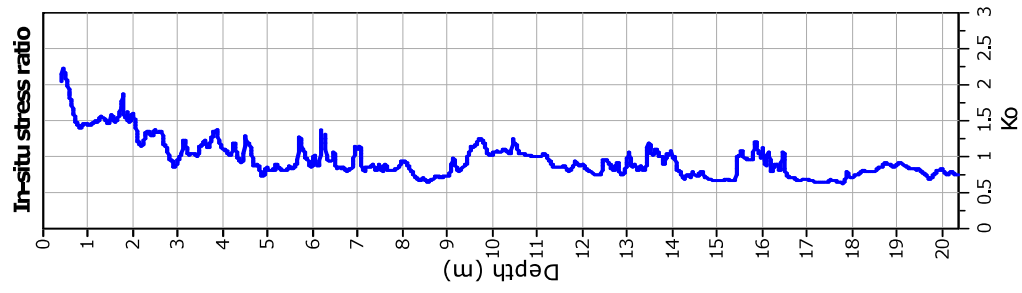
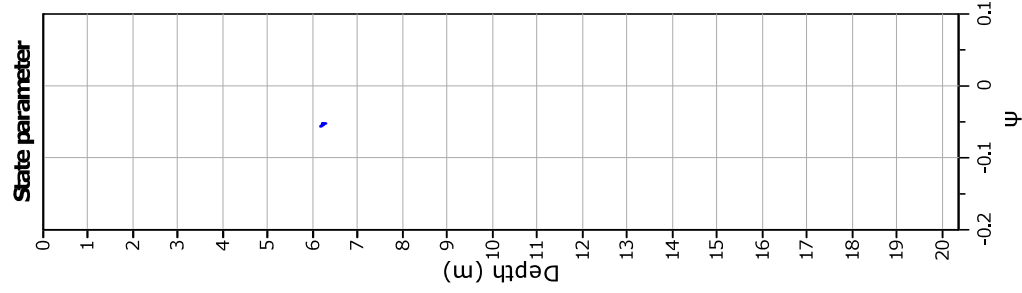
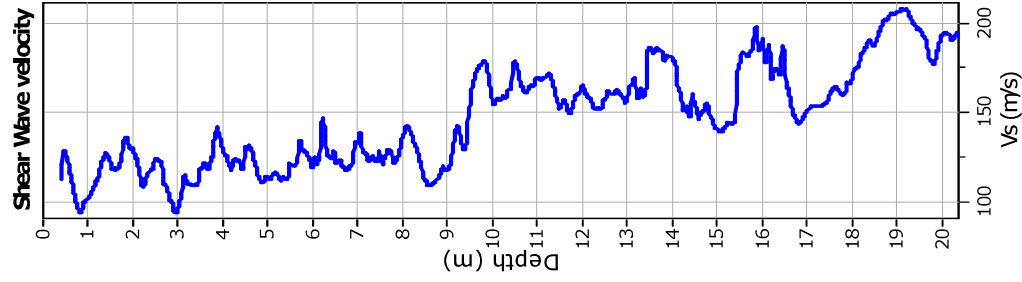
—●— User defined estimation data



Calculation parameters

Constrained modulus: Based on variable α/β using I_c and Q_m (Robertson, 2009)
Go: Based on variable α/β using I_c (Robertson, 2009)
Undrained shear strength cone factor for clays, N_{kt} : 14

OCR factor for clays, N_{kt} : 0.33
—●— User defined estimation data
—●— Flat Dilatometer Test data



Calculation parameters

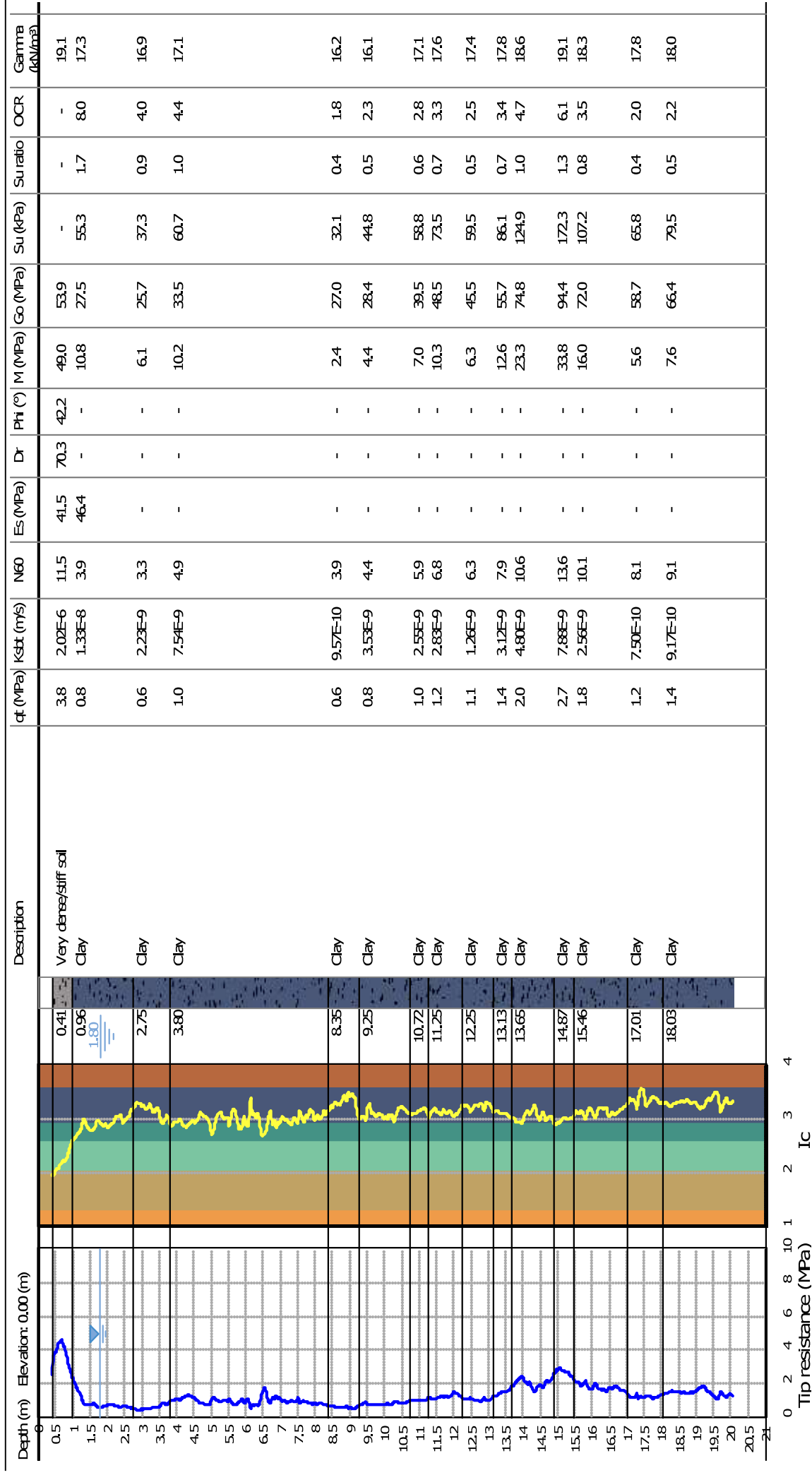
Soil Sensitivity factor, N_s : 7.00

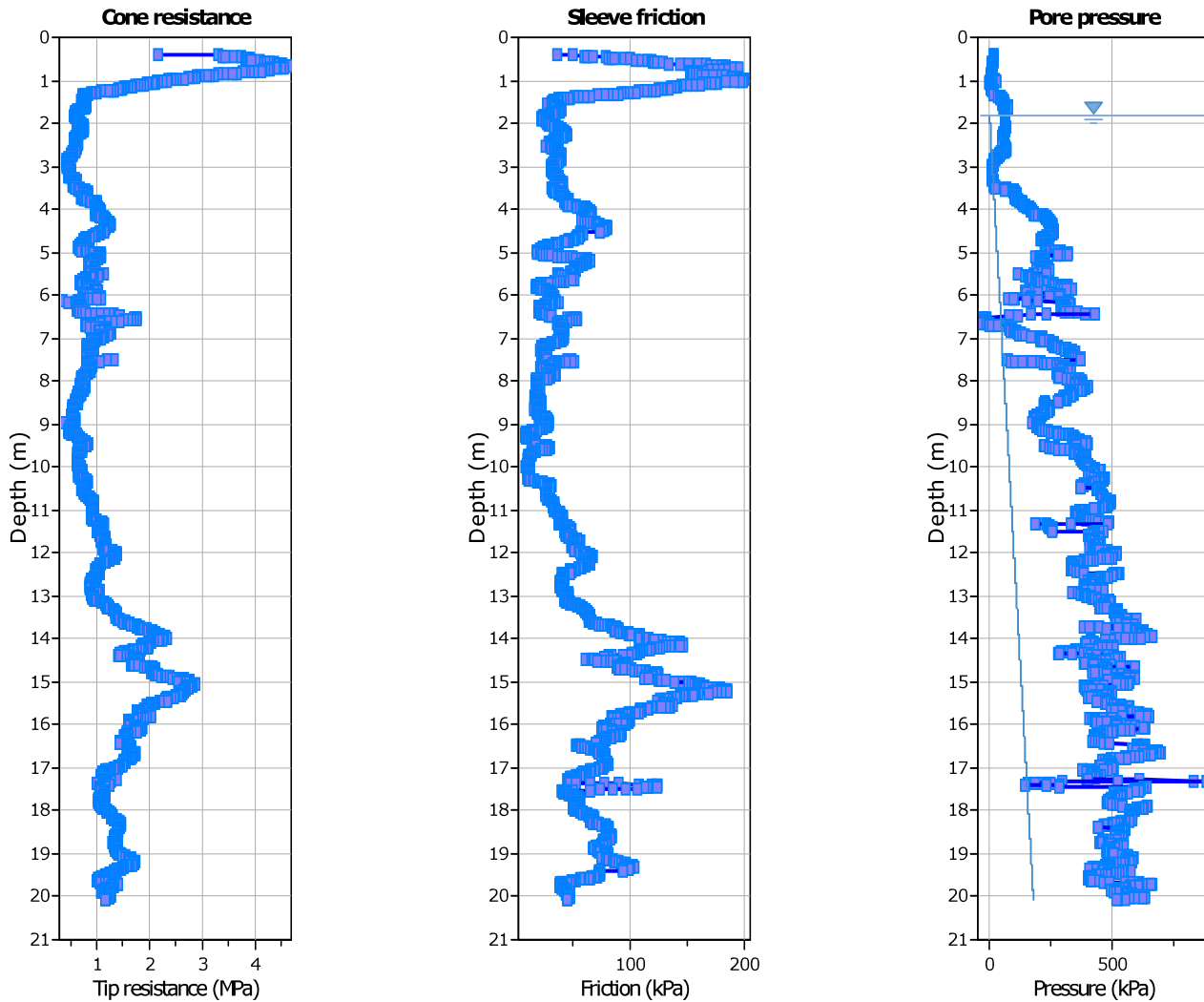
—●— User defined estimation data



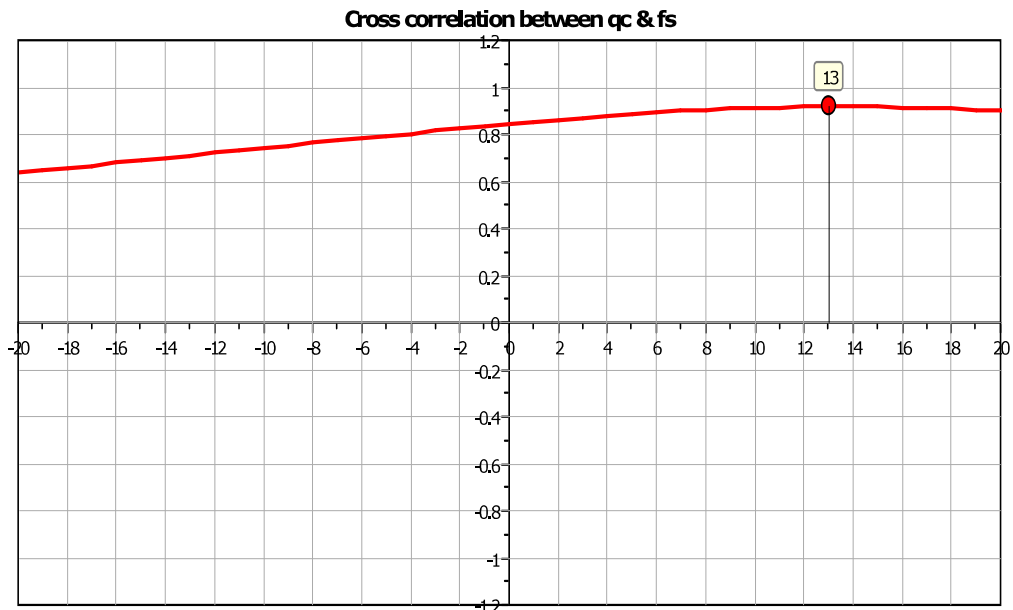
UFFICI: via Per Modena, 12 - 41051 Castelnuovo R. (MO) SEDE LEGALE: via C. Costa, 182 - 41123 Modena (MO)
tel. 059/3967169; 059/828367 - FAX 059/5960176 p. Iva e C.F. 02981500362
Email: info@geogroupmodena.it Pec: geo.group@winpec.it Sito: www.geogroupmodena.it
Operatore APR certificato ENAC per operazioni non critiche – Rif. ENAC 18627

CPTu 3

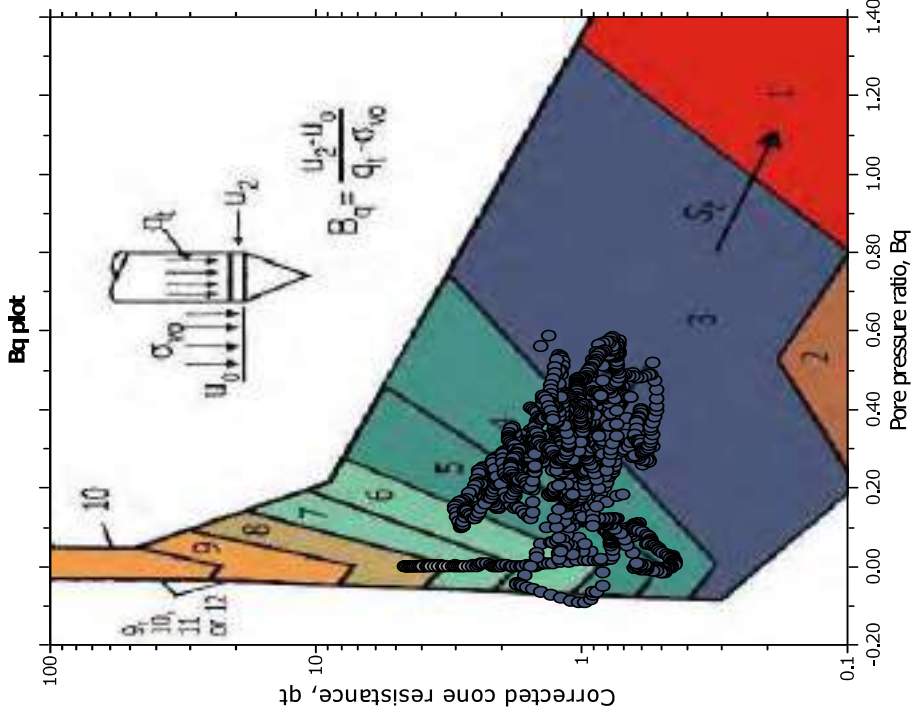
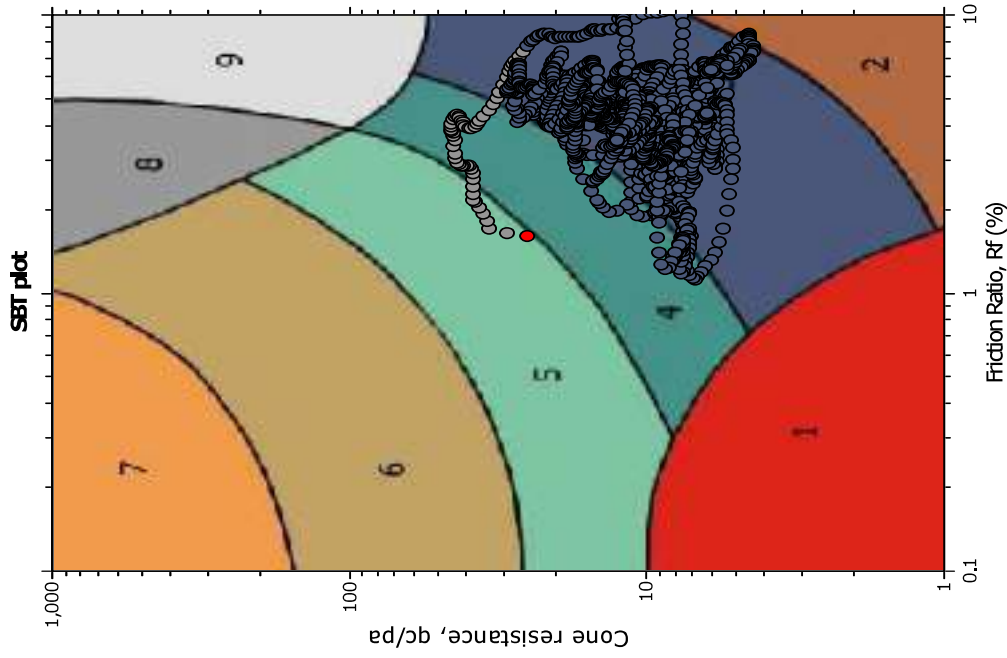




The plot below presents the cross correlation coefficient between the raw q_c and f_s values (as measured on the field). X axes presents the lag distance (one lag is the distance between two successive CPT measurements).



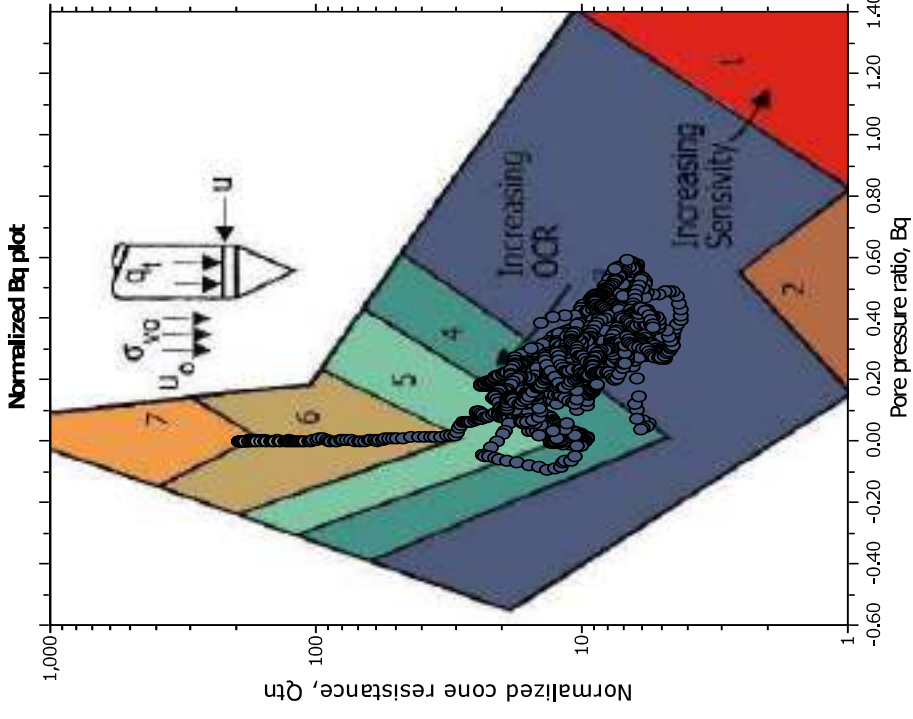
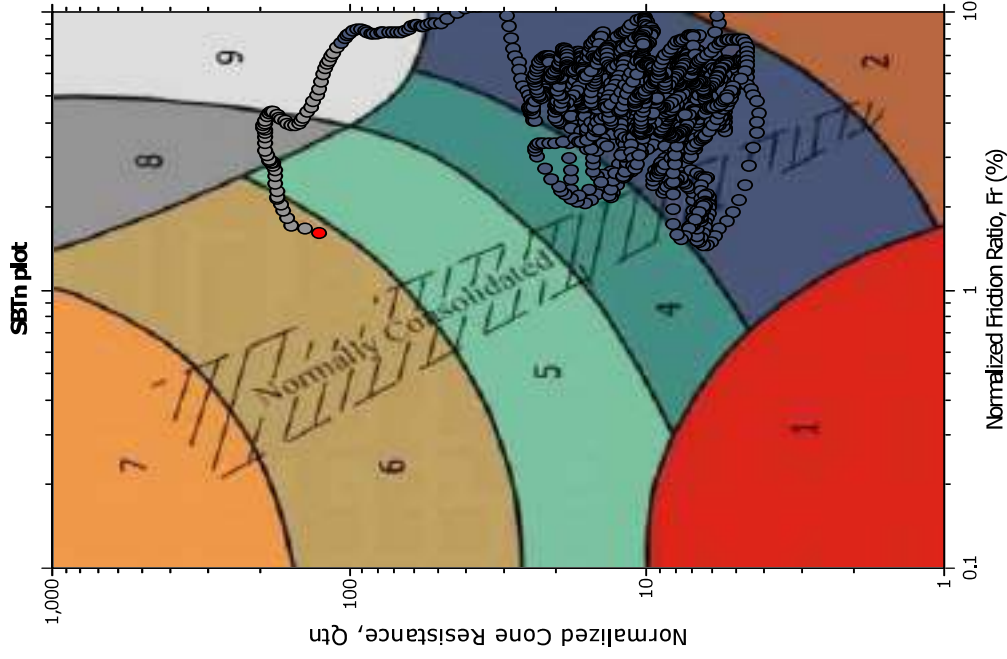
SBT - Bq plots



SBT legend

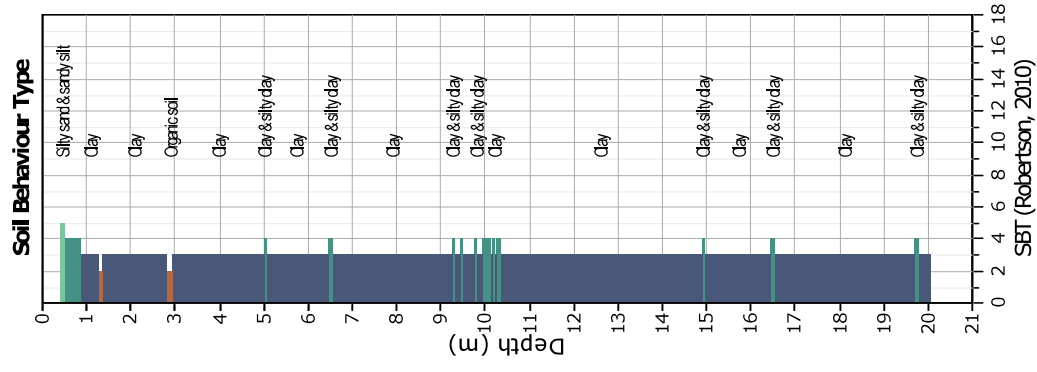
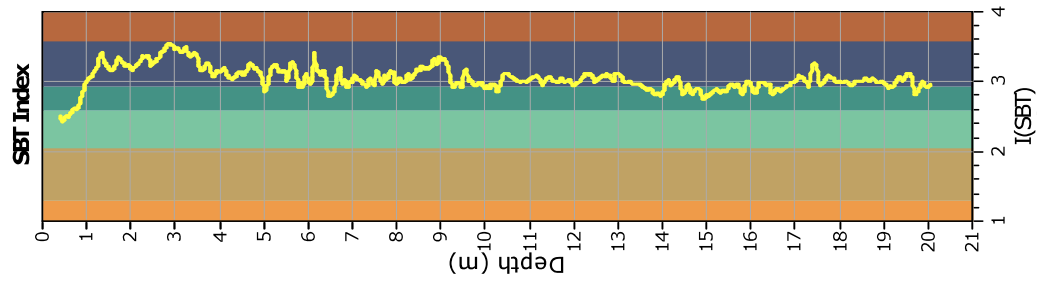
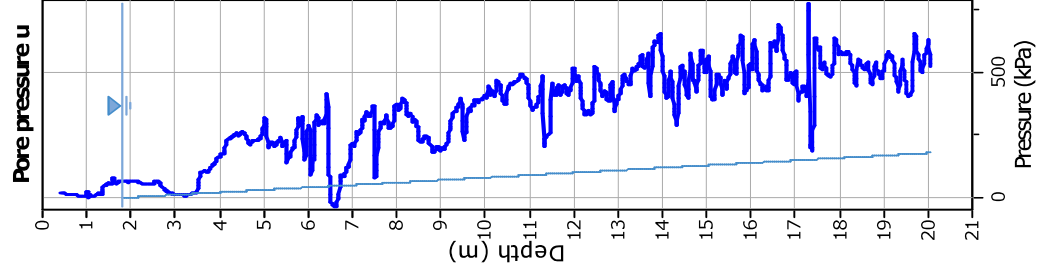
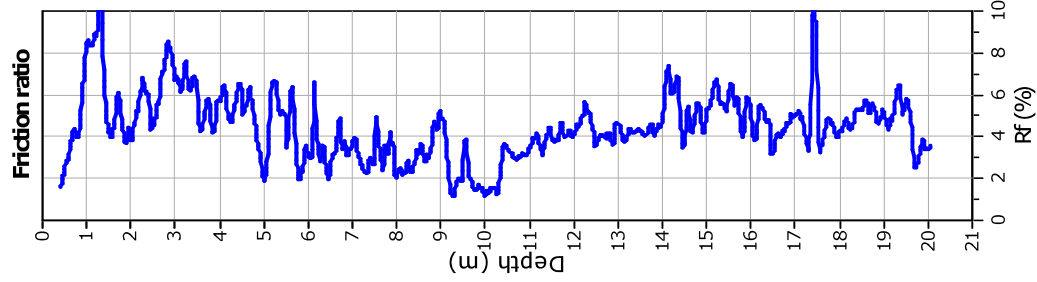
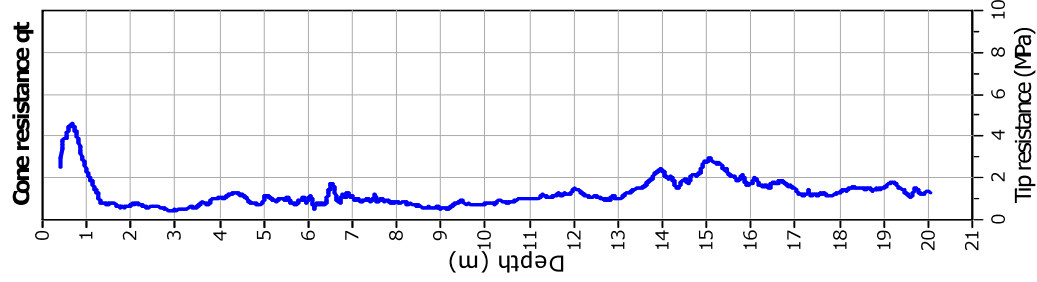
- 1. Sensitive fine grained
- 2. Organic material
- 3. Clay to silty clay
- 4. Clayey silt to silty clay
- 5. Silty sand to sandy silt
- 6. Clean sand to silty sand
- 7. Gravely sand to sand
- 8. Very stiff sand to clayey sand
- 9. Very stiff fine grained

SBT - Bq plots (normalized)

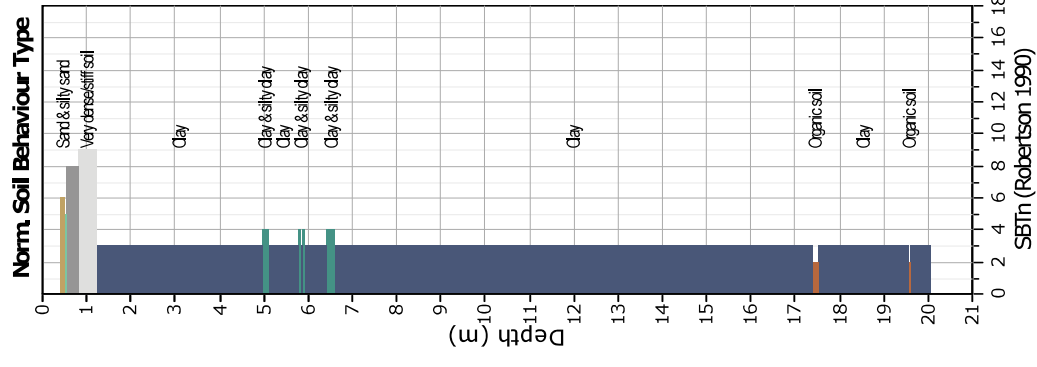
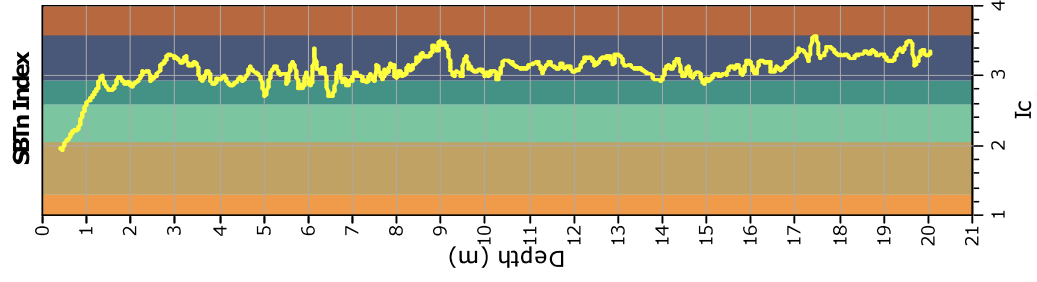
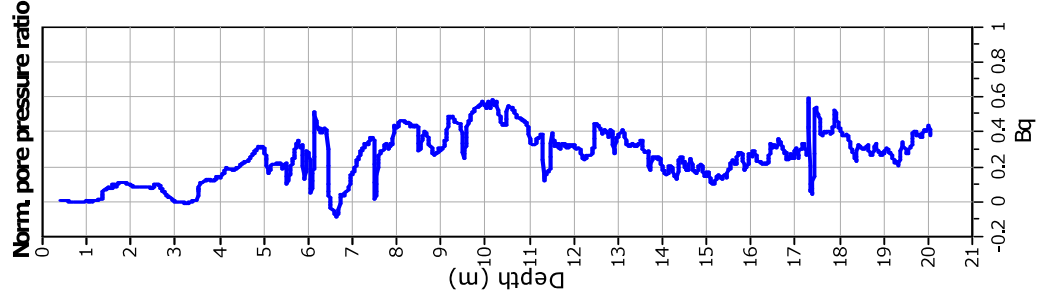
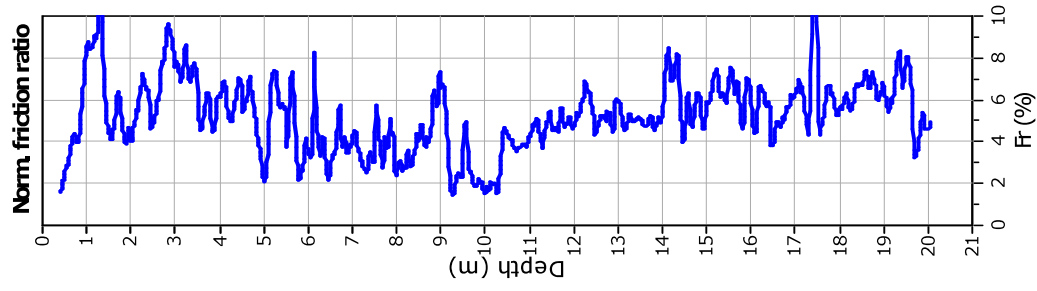
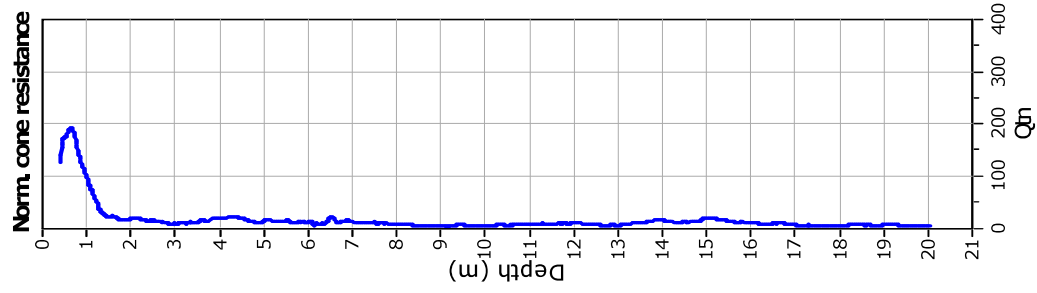


SBTn legend

- 1. Sensitive fine grained
- 2. Organic material
- 3. Clay to silty clay
- 4. Clayey silt to silty clay
- 5. Silty sand to sandy silt
- 6. Clean sand to silty sand
- 7. Gravely sand to sand
- 8. Very stiff sand to clayey sand
- 9. Very stiff fine grained

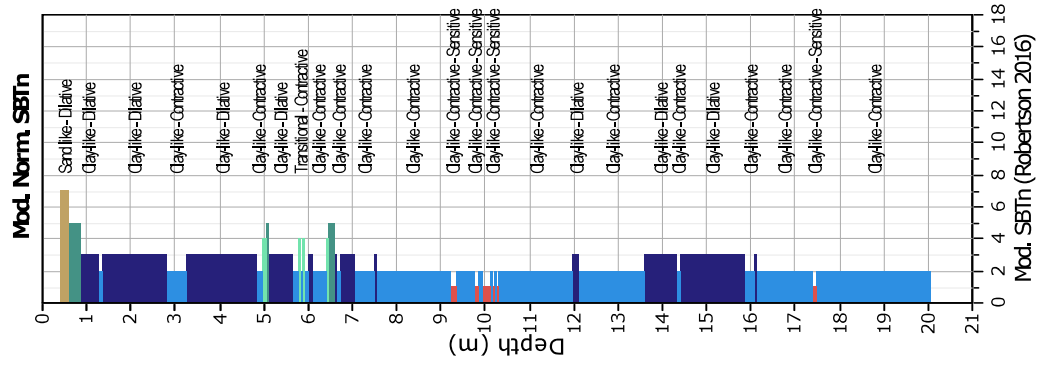
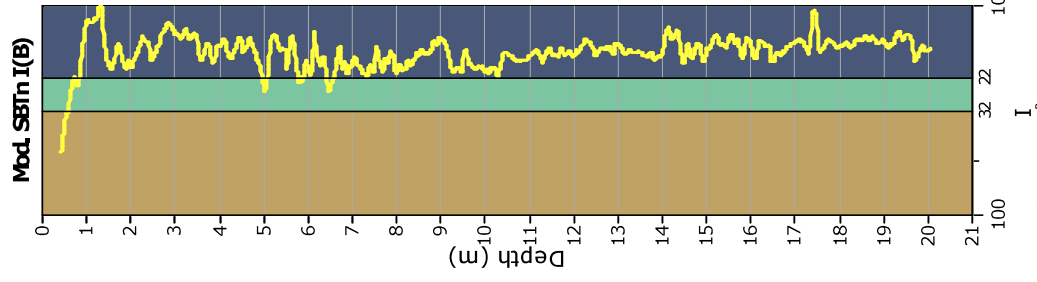
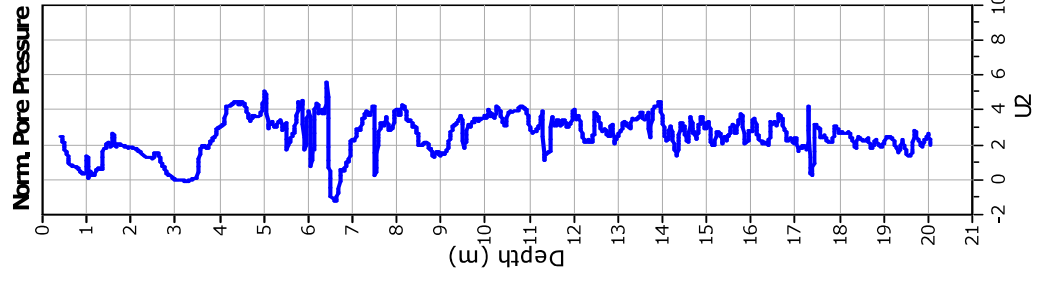
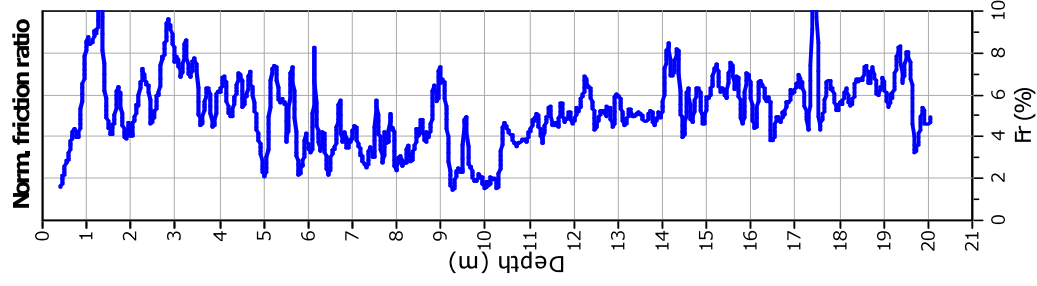
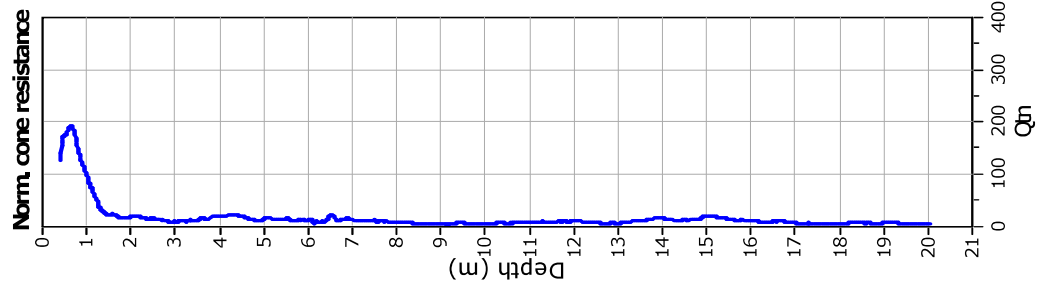


- SBT legend**
- 1. Sensitive fine grained
 - 2. Organic material
 - 3. Clay to silty clay
 - 4. Clayey silt to silty clay
 - 5. Silty sand to sandy silt
 - 6. Clean sand to silty sand
 - 7. Gravely sand to sand
 - 8. Very stiff sand to clayey sand
 - 9. Very stiff fine grained



SBTn legend

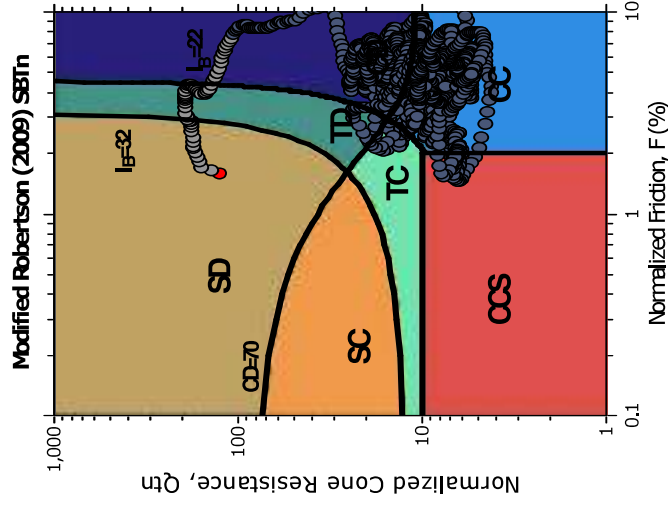
- 1. Sensitive fine grained
- 2. Organic material
- 3. Clay to silty clay
- 4. Clayey silt to silty clay
- 5. Silty sand to sandy silt
- 6. Clean sand to silty sand
- 7. Gravely sand to sand
- 8. Very stiff sand to clayey sand
- 9. Very stiff fine grained



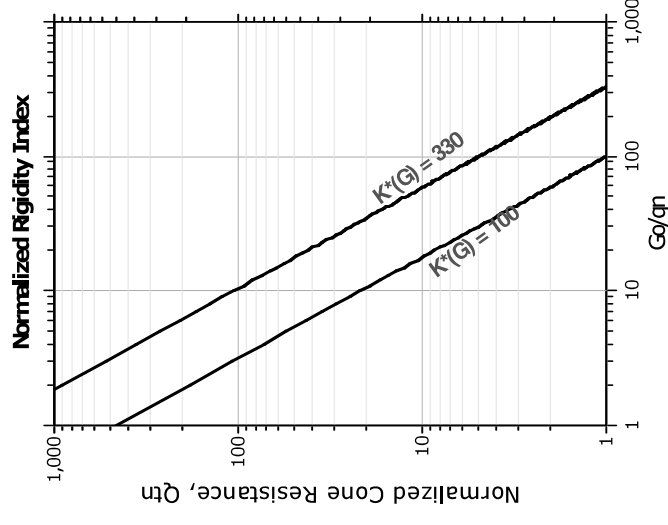
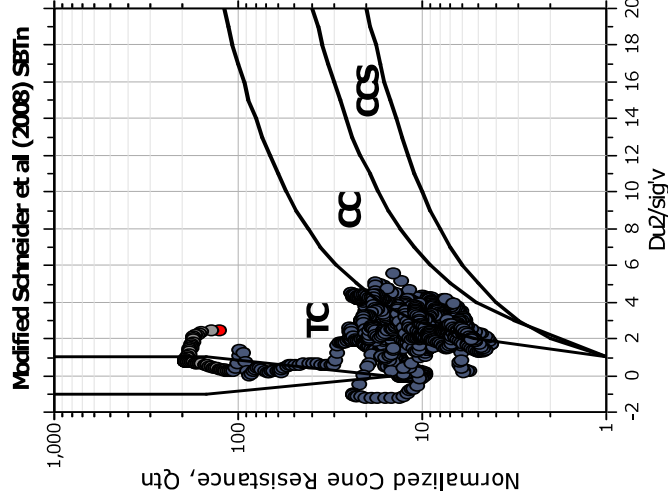
Mod. SBTn legend

- 1. CCS: ClayLike - Contractive, Sensitive
- 2. CC: Clay-like - Contractive
- 3. CD: Clay-Like: Dilative
- 4. TC: Transitional - Contractive
- 5. TD: Transitional - Dilative
- 6. SC: Sand-like - Contractive
- 7. SD: Sand-like - Dilative

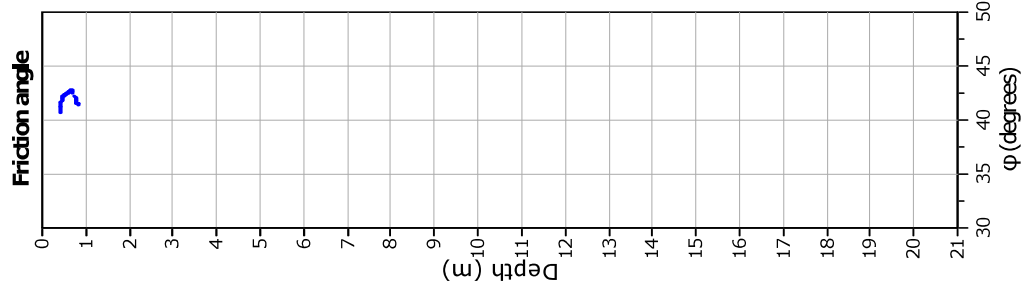
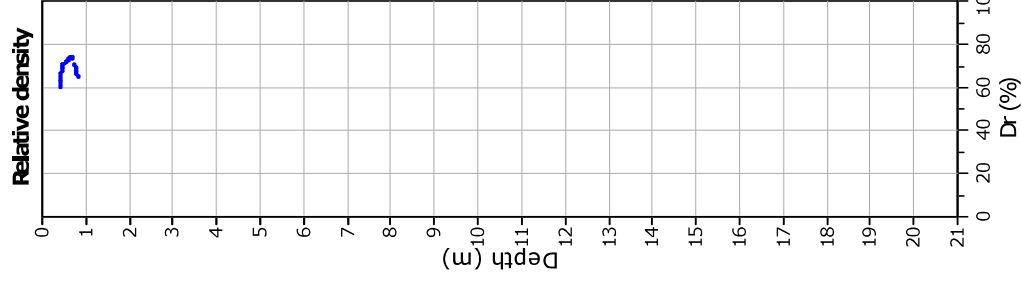
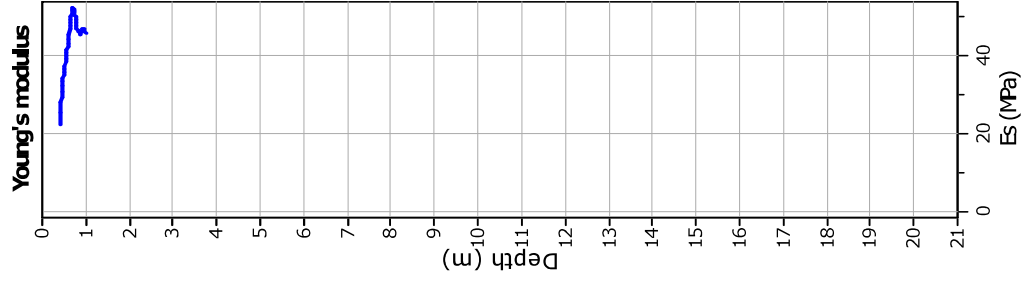
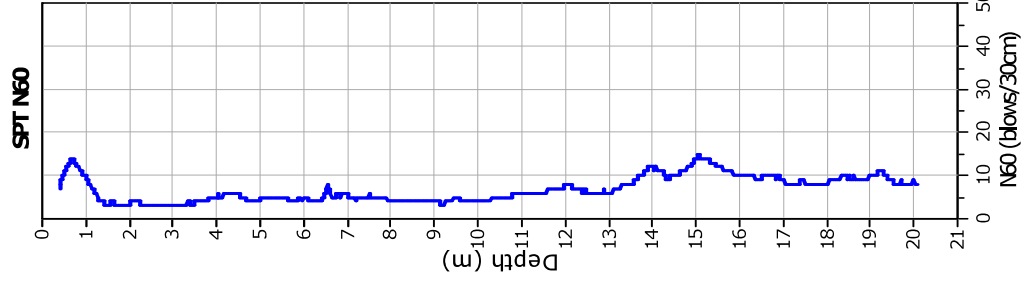
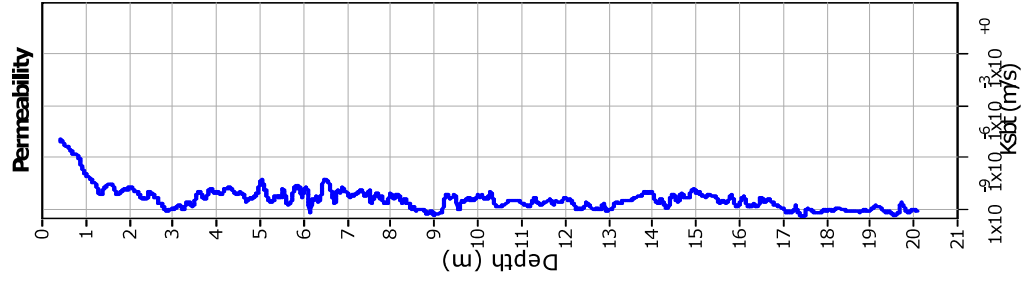
Updated SBTn plots



- CCS: Clay-like - Contractive - Sensitive
- CC: Clay-like - Contractive
- CD: Clay-like - Dilative
- TC: Transitional - Contractive
- TD: Transitional - Dilative
- SC: Sand-like - Contractive
- SD: Sand-like - Dilative



$K(G) > 330$: Soils with significant microstructure (e.g. age/cementation)



Calculation parameters

Permeability: Based on SBT_n

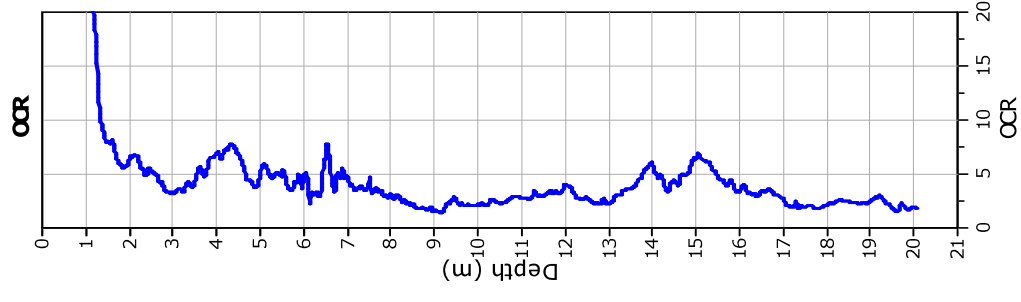
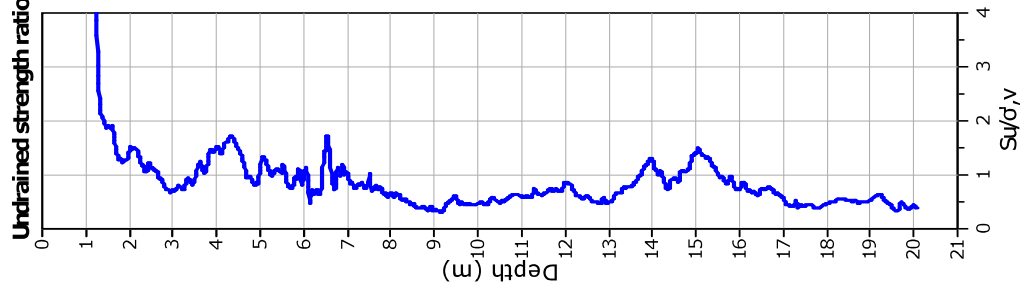
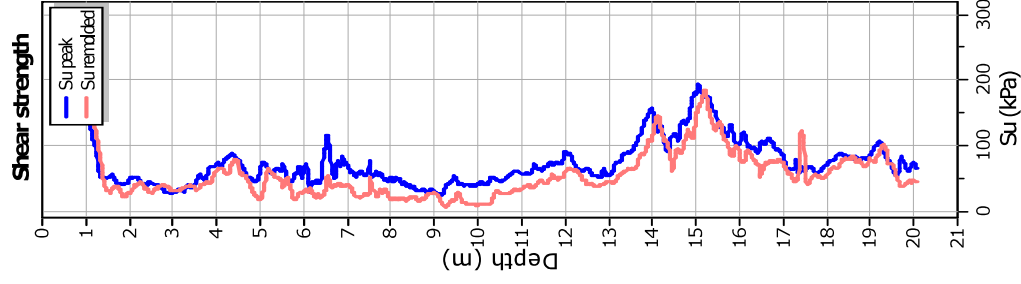
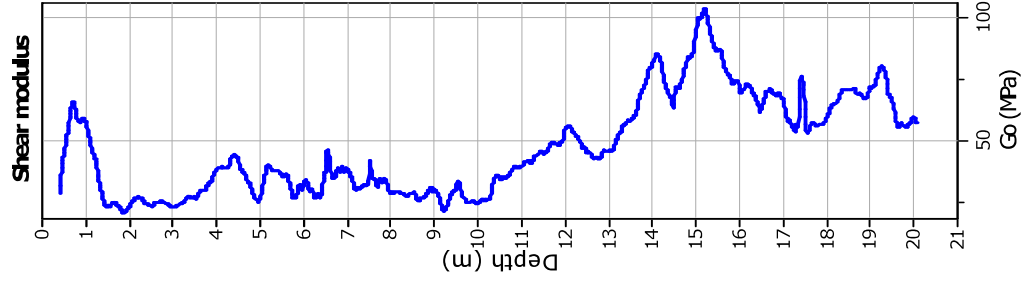
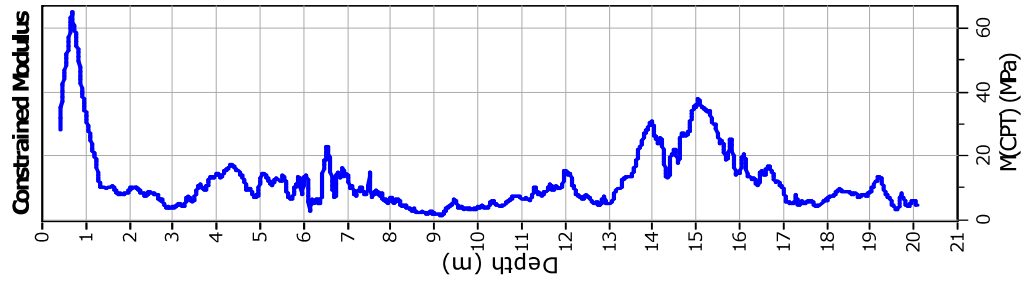
SPT N_{60} : Based on I_c and q_t

Young's modulus: Based on variable alpha using I_c (Robertson, 2009)

Relative density constant, C_{Dr} : 350.0

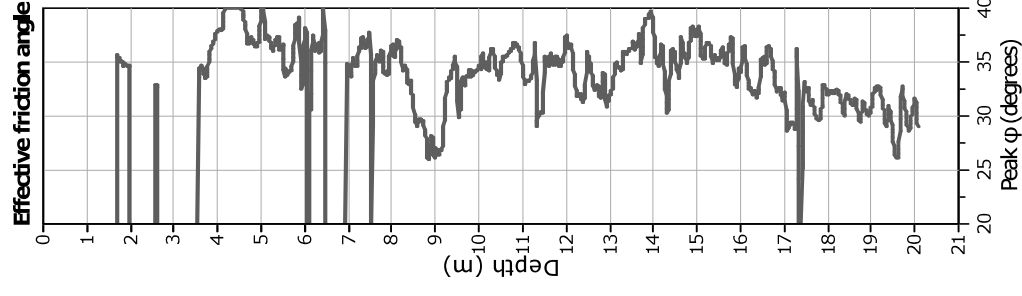
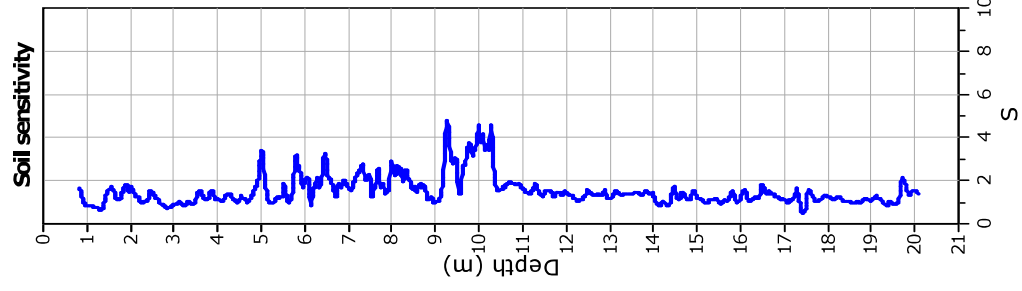
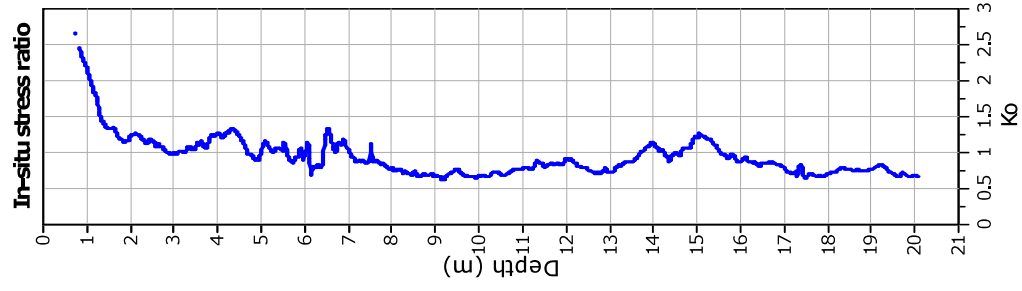
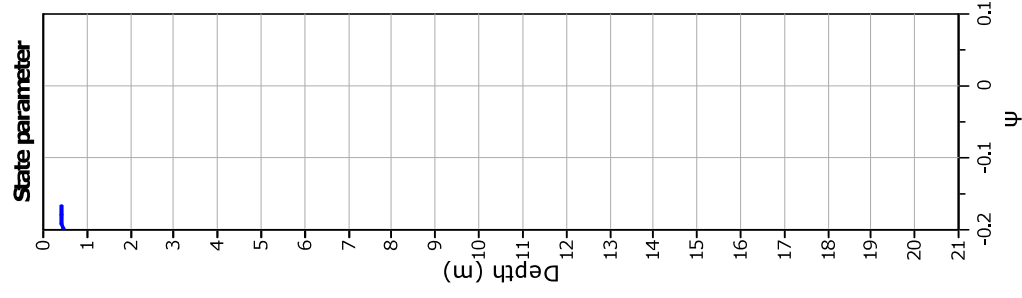
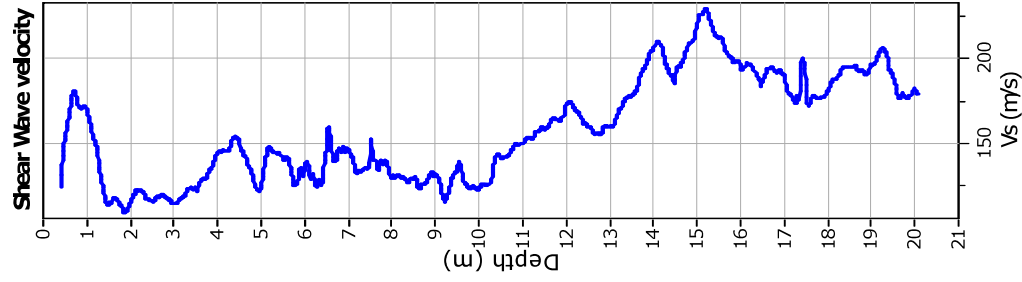
Phi: Based on Kulhawy & Mayne (1990)

—●— User defined estimation data



Calculation parameters

Constrained modulus: Based on variable α/β using I_c and Q_m (Robertson, 2009) OCR factor for clays, N_{kt} : 0.33
 Go: Based on variable α/β using I_c (Robertson, 2009) — User defined estimation data
 Undrained shear strength cone factor for clays, N_{kt} : 14 — Flat Dilatometer Test data



Calculation parameters

Soil Sensitivity factor, N_s : 7.00

—●— User defined estimation data

ALL.2

Verifica alla liquefazione



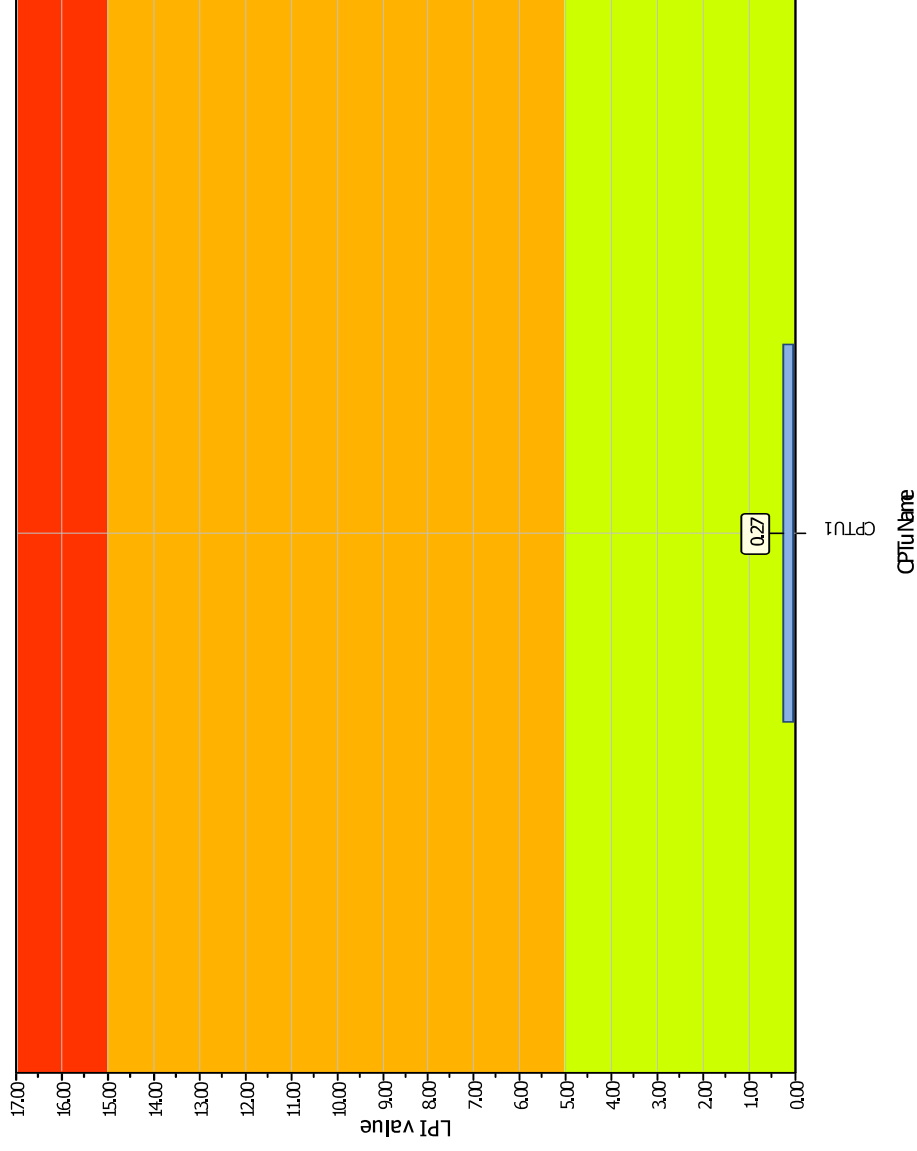
UFFICI: via Per Modena, 12 - 41051 Castelnuovo R. (MO) SEDE LEGALE: via C. Costa, 182 - 41123 Modena (MO)
tel. 059/3967169; 059/828367 - FAX 059/5960176 p. Iva e C.F. 02981500362
Email: info@geogroupmodena.it Pec: geo.group@winpec.it Sito: www.geogroupmodena.it
Operatore APR certificato ENAC per operazioni non critiche – Rif. ENAC 18627

Verifica alla liquefazione su CPTu 1

Project title : Studio del terreno di fondazione

Location : Campogalliano, TRANSMEC

Overall Liquefaction Potential Index report



LPI color scheme

- Very high risk
- High risk
- Low risk

Basic statistics

Total CPT number: 1
 100.00% low risk
 0.00% high risk
 0.00% very high risk

LIQUEFACTION ANALYSIS REPORT

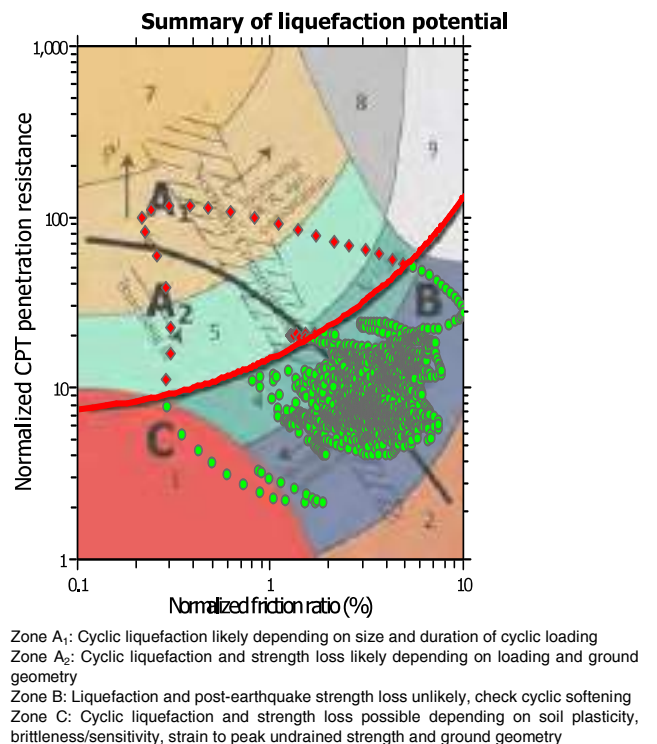
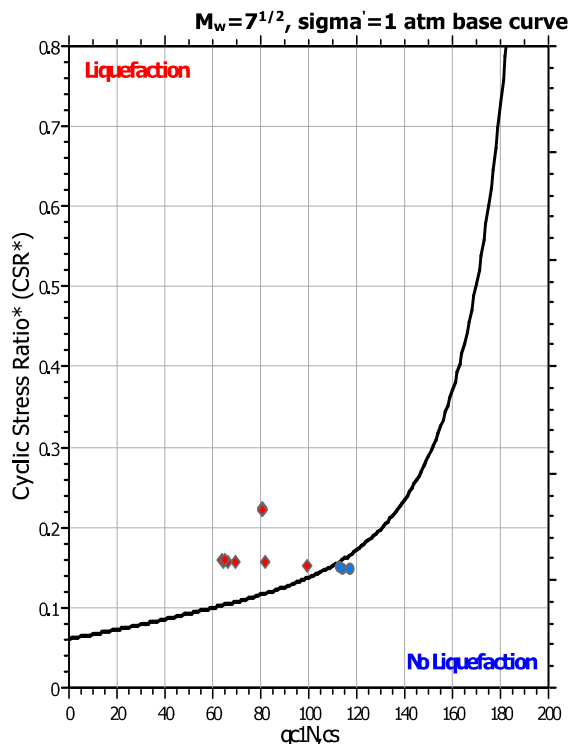
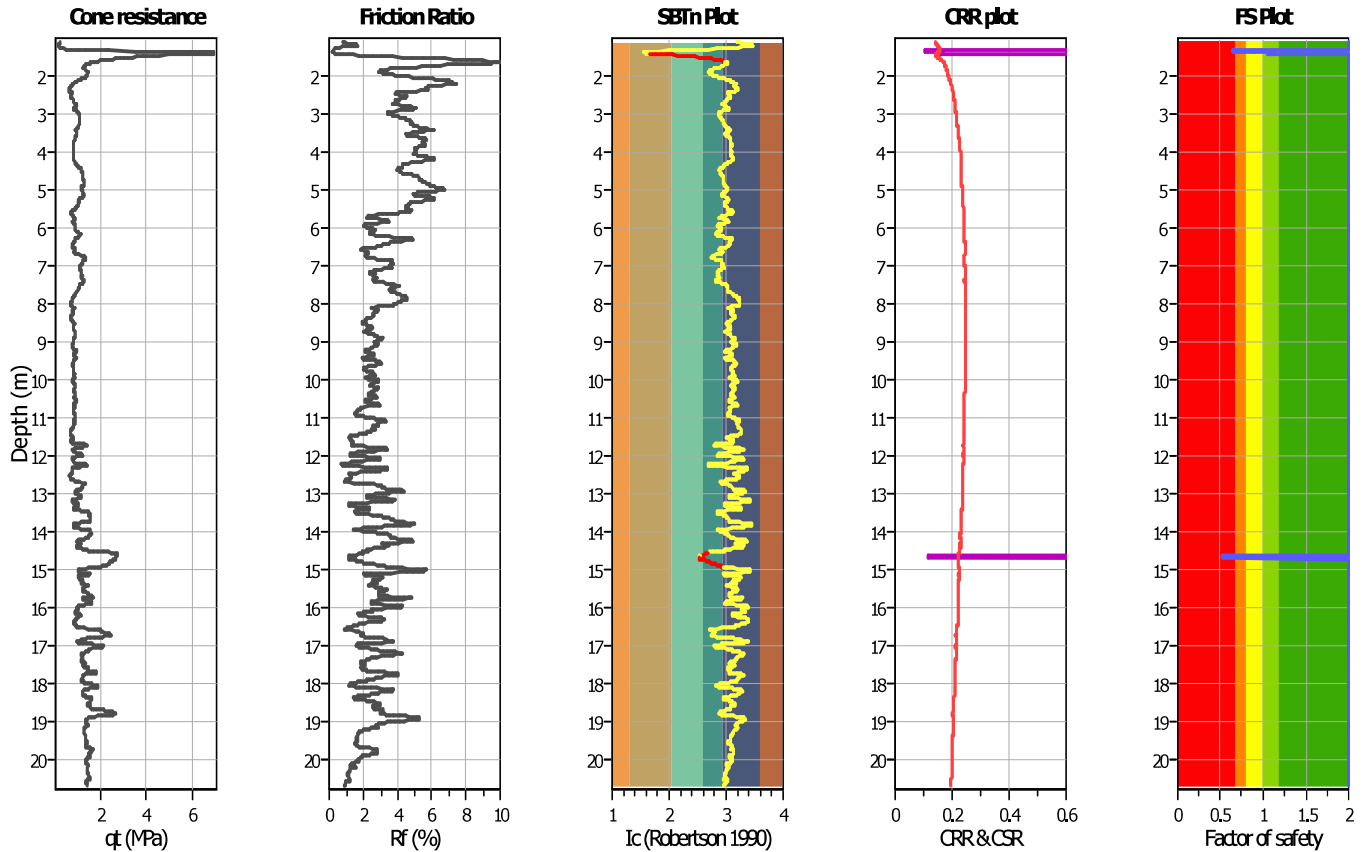
Project title : Studio del terreno di fondazione

Location : Campogalliano, TRANSMEC

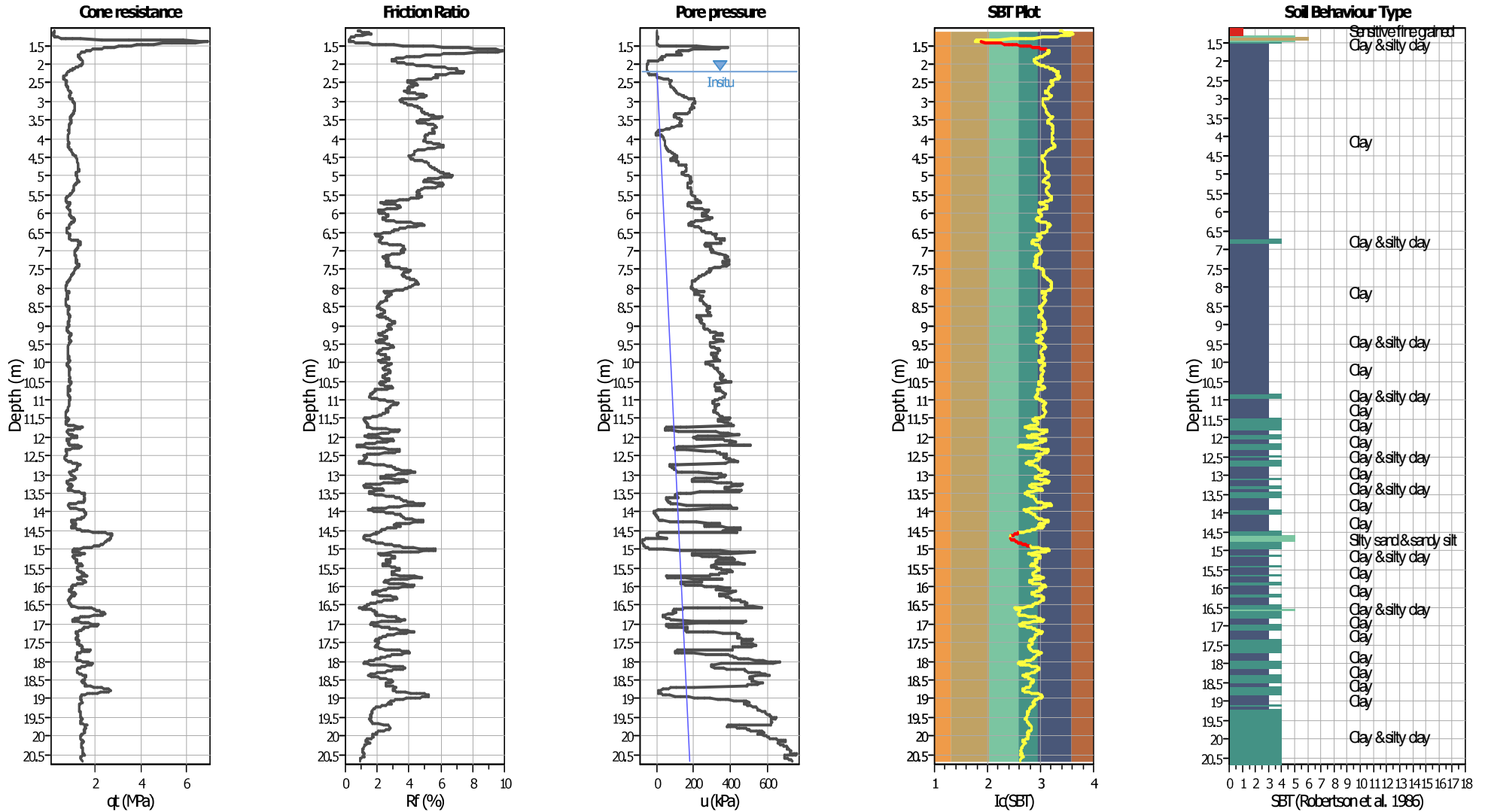
CPT file : CPTU1

Input parameters and analysis data

Analysis method:	B&I (2014)	G.W.T. (in-situ):	2.20 m	Use fill:	No	Clay like behavior	
Fines correction method:	B&I (2014)	G.W.T. (earthq.):	1.00 m	Fill height:	N/A	applied:	Sands only
Points to test:	Based on Ic value	Average results interval:	3	Fill weight:	N/A	Limit depth applied:	Yes
Earthquake magnitude M_w :	6.14	Ic cut-off value:	2.60	Trans. detect. applied:	Yes	Limit depth:	20.00 m
Peak ground acceleration:	0.24	Unit weight calculation:	Based on SBT	K_σ applied:	Yes	MSF method:	Method



CPT basic interpretation plots



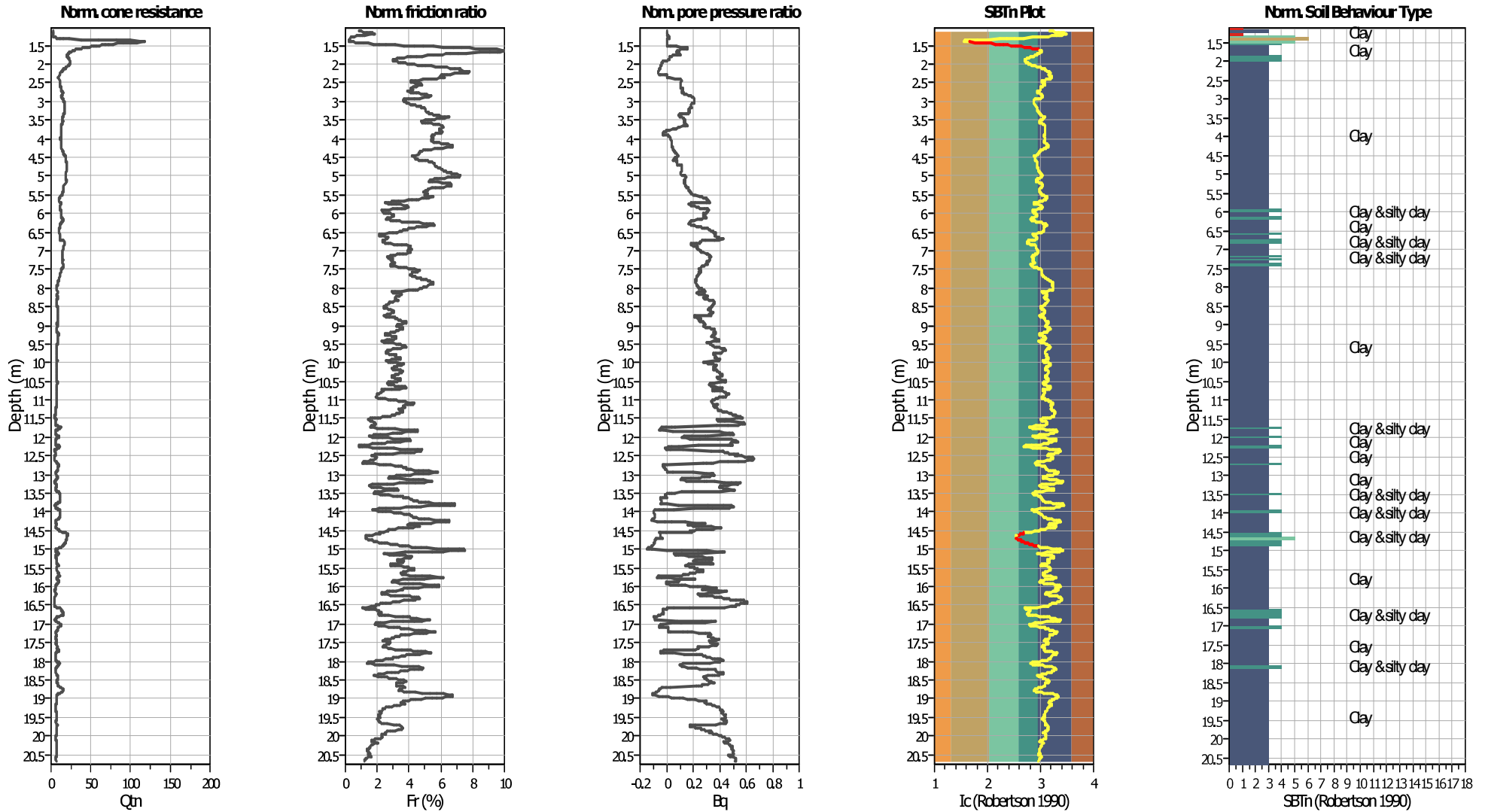
Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (erthq.):	1.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on I_c value	I_c cut-off value:	2.60	K_p applied:	Yes
Earthquake magnitude M_w :	6.14	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sands only
Peak ground acceleration:	0.24	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.20 m	Fill height:	N/A	Limit depth:	20.00 m

SBT legend

1. Sensitive fine grained	4. Clayey silt to silty	7. Gravely sand to sand
2. Organic material	5. Silty sand to sandy silt	8. Very stiff sand to
3. Clay to silty clay	6. Clean sand to silty sand	9. Very stiff fine grained

CPT basic interpretation plots (normalized)



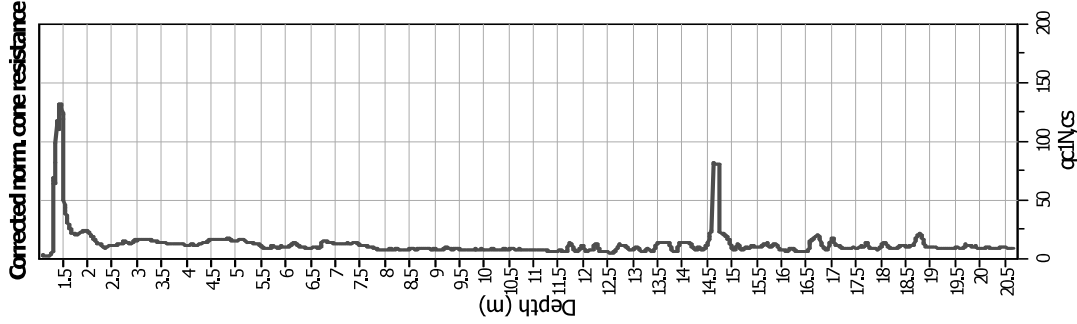
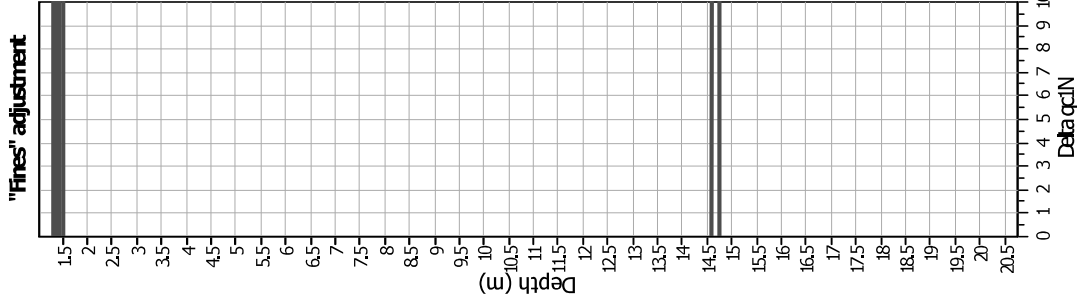
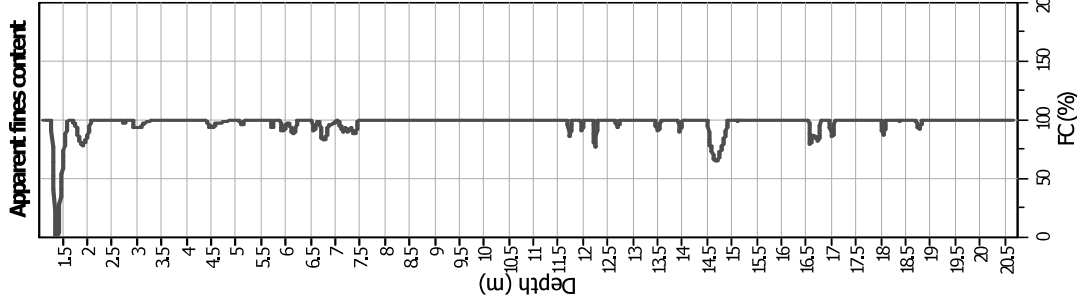
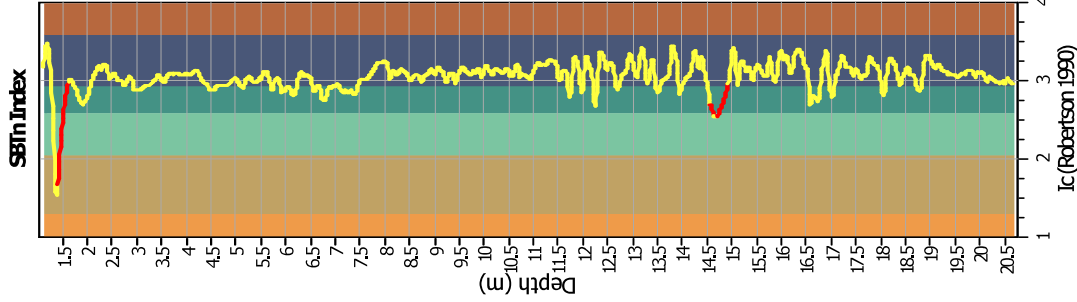
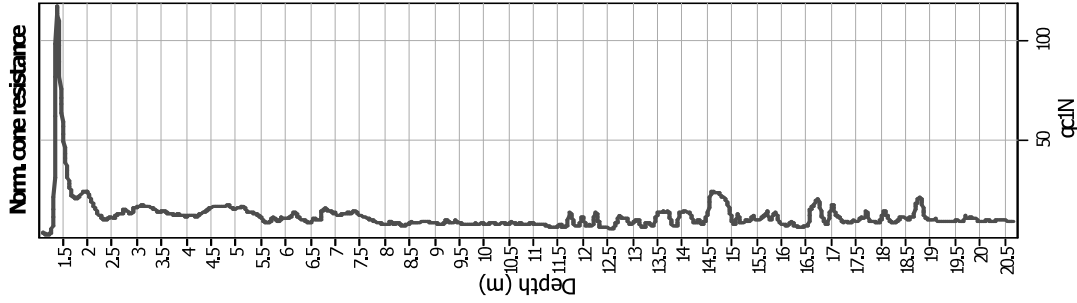
Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (erthq.):	1.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_p applied:	Yes
Earthquake magnitude M_w :	6.14	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sands only
Peak ground acceleration:	0.24	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.20 m	Fill height:	N/A	Limit depth:	20.00 m

SBTn legend

■ 1. Sensitive fine grained	■ 4. Clayey silt to silty	■ 7. Gravely sand to sand
■ 2. Organic material	■ 5. Silty sand to sandy silt	■ 8. Very stiff sand to
■ 3. Clay to silty clay	■ 6. Clean sand to silty sand	■ 9. Very stiff fine grained

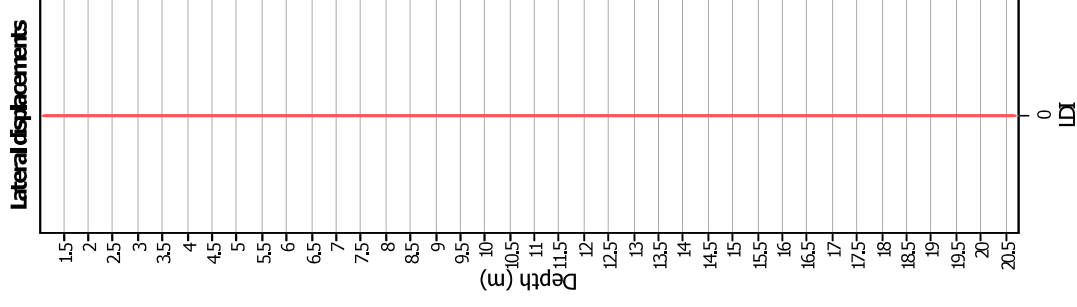
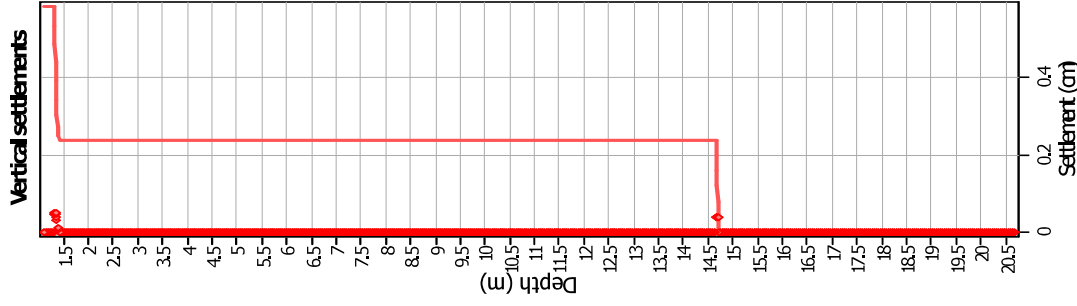
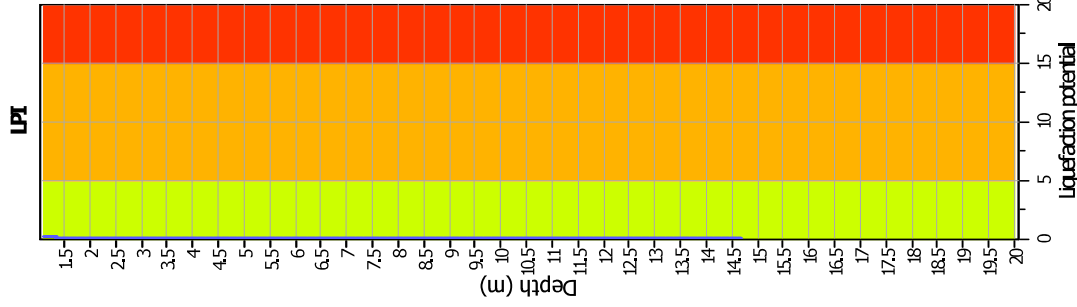
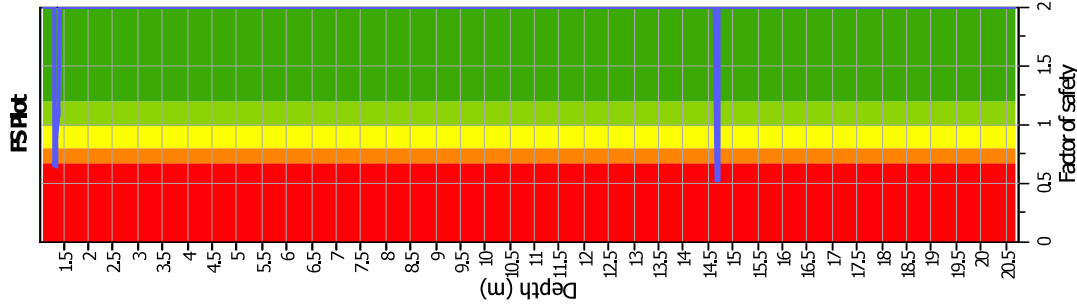
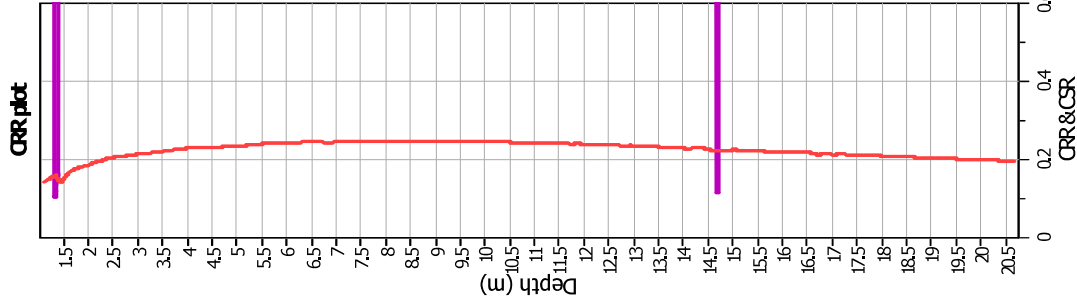
Liquefaction analysis overall plots (intermediate results)



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	1.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _r applied:	Yes
Earthquake magnitude M _w :	6.14	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sands only
Peak ground acceleration:	0.24	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.20 m	Fill height:	N/A	Limit depth:	20.00 m

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWL (earthq.):	1.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _v applied:	Yes
Earthquake magnitude M _w :	6.14	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sands only
Peak ground acceleration:	0.24	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.20 m	Fill height:	N/A	Limit depth:	20.00 m

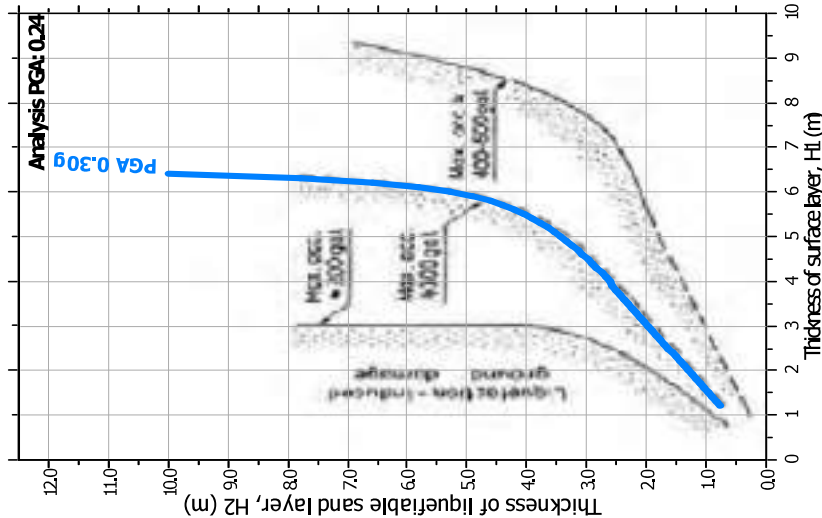
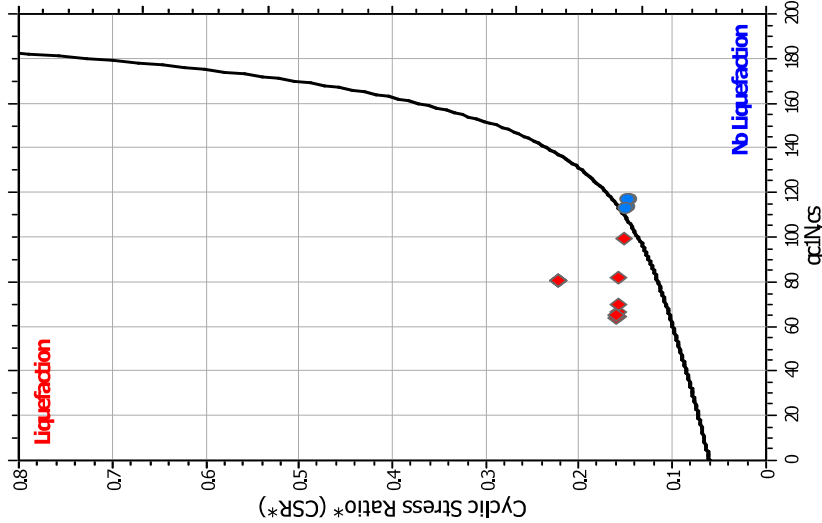
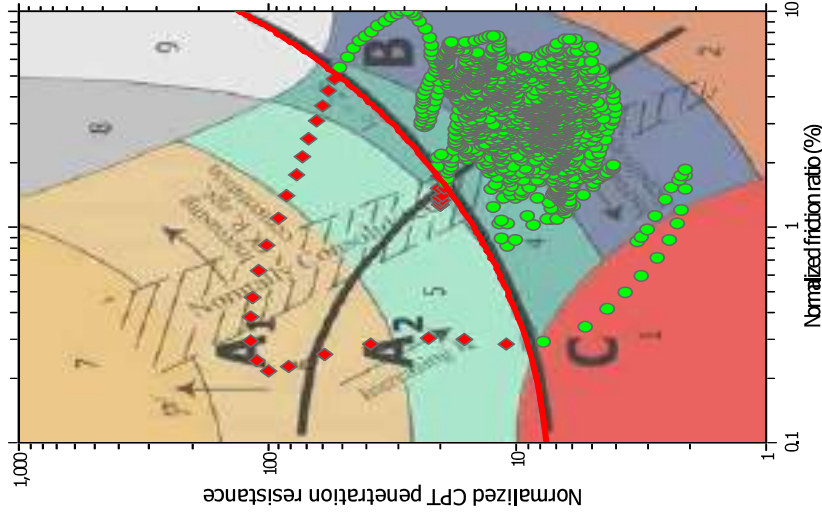
F.S. color scheme

Red	Almost certain it will liquefy
Orange	Very likely to liquefy
Yellow	Liquefaction and no liq. are equally likely
Light Green	Unlike to liquefy
Dark Green	Almost certain it will not liquefy

LPI color scheme

Red	Very high risk
Orange	High risk
Light Green	Low risk

Liquefaction analysis summary plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GW (earthq.):	1.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_r applied:	Yes
Earthquake magnitude M_w :	6.14	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sands only
Peak ground acceleration:	0.24	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	2.20 m	Fill height:	N/A	Limit depth:	20.00 m



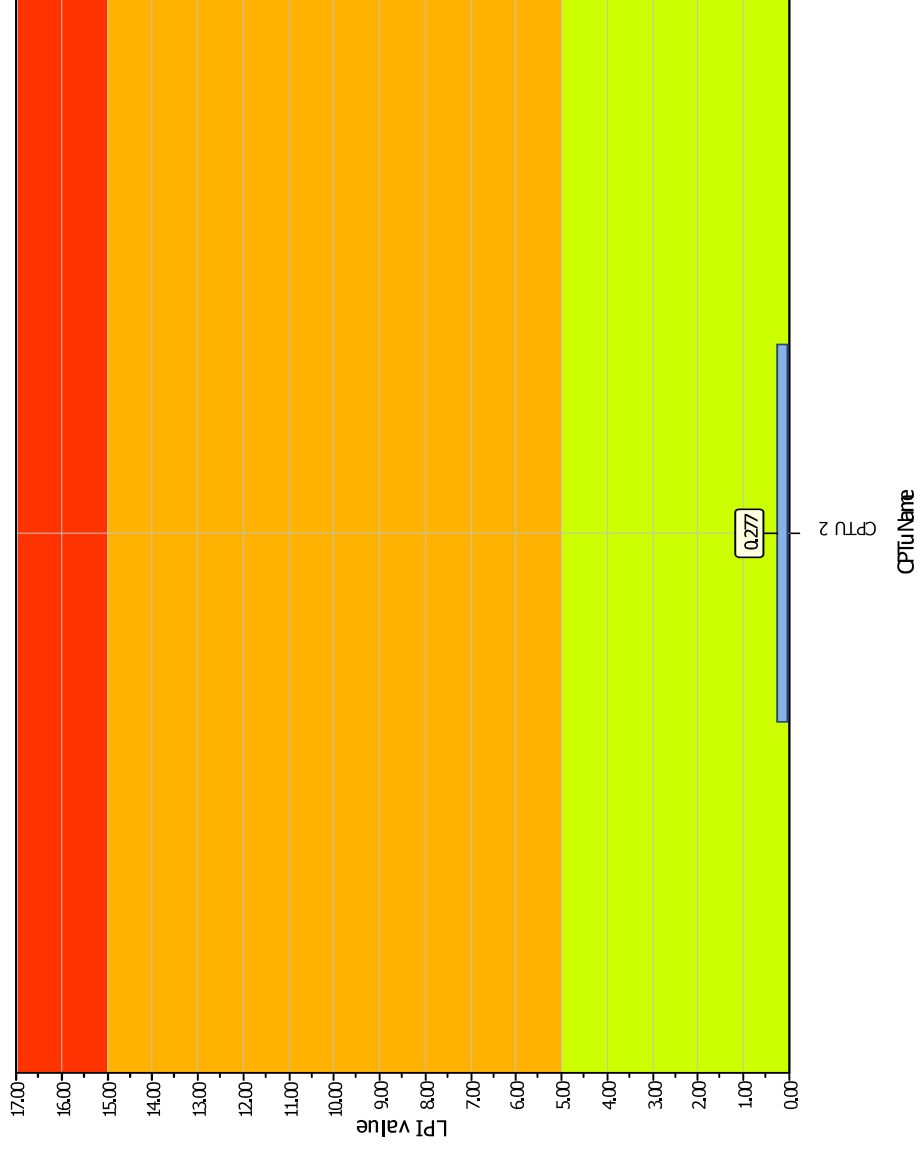
UFFICI: via Per Modena, 12 - 41051 Castelnuovo R. (MO) SEDE LEGALE: via C. Costa, 182 - 41123 Modena (MO)
tel. 059/3967169; 059/828367 - FAX 059/5960176 p. Iva e C.F. 02981500362
Email: info@geogroupmodena.it Pec: geo.group@winpec.it Sito: www.geogroupmodena.it
Operatore APR certificato ENAC per operazioni non critiche – Rif. ENAC 18627

Verifica alla liquefazione su CPTu 2

Project title :

Location : Campogalliano, TRANSMEC

Overall Liquefaction Potential Index report



LPI color scheme

- Very high risk
- High risk
- Low risk

Basic statistics

Total CPT number: 1
 100.00% low risk
 0.00% high risk
 0.00% very high risk

LIQUEFACTION ANALYSIS REPORT

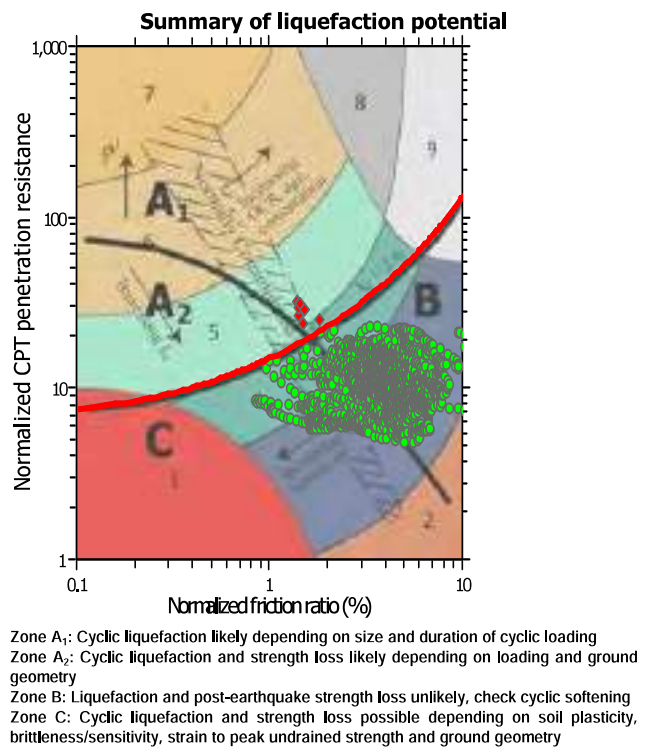
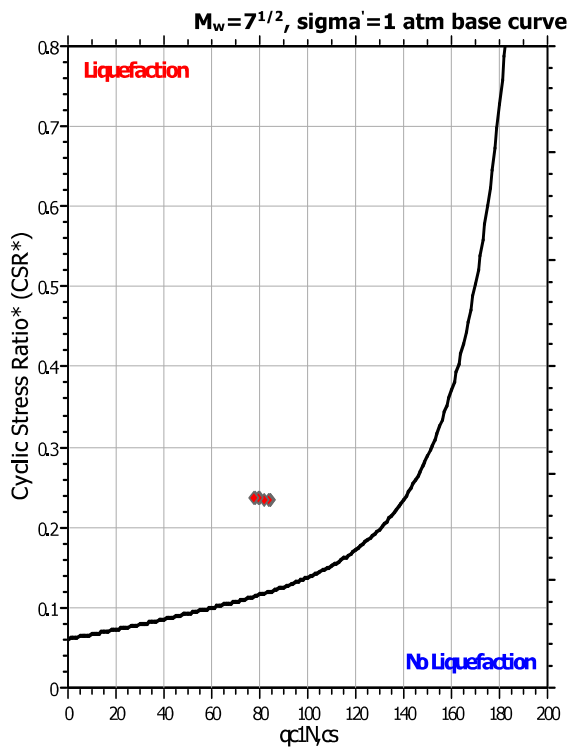
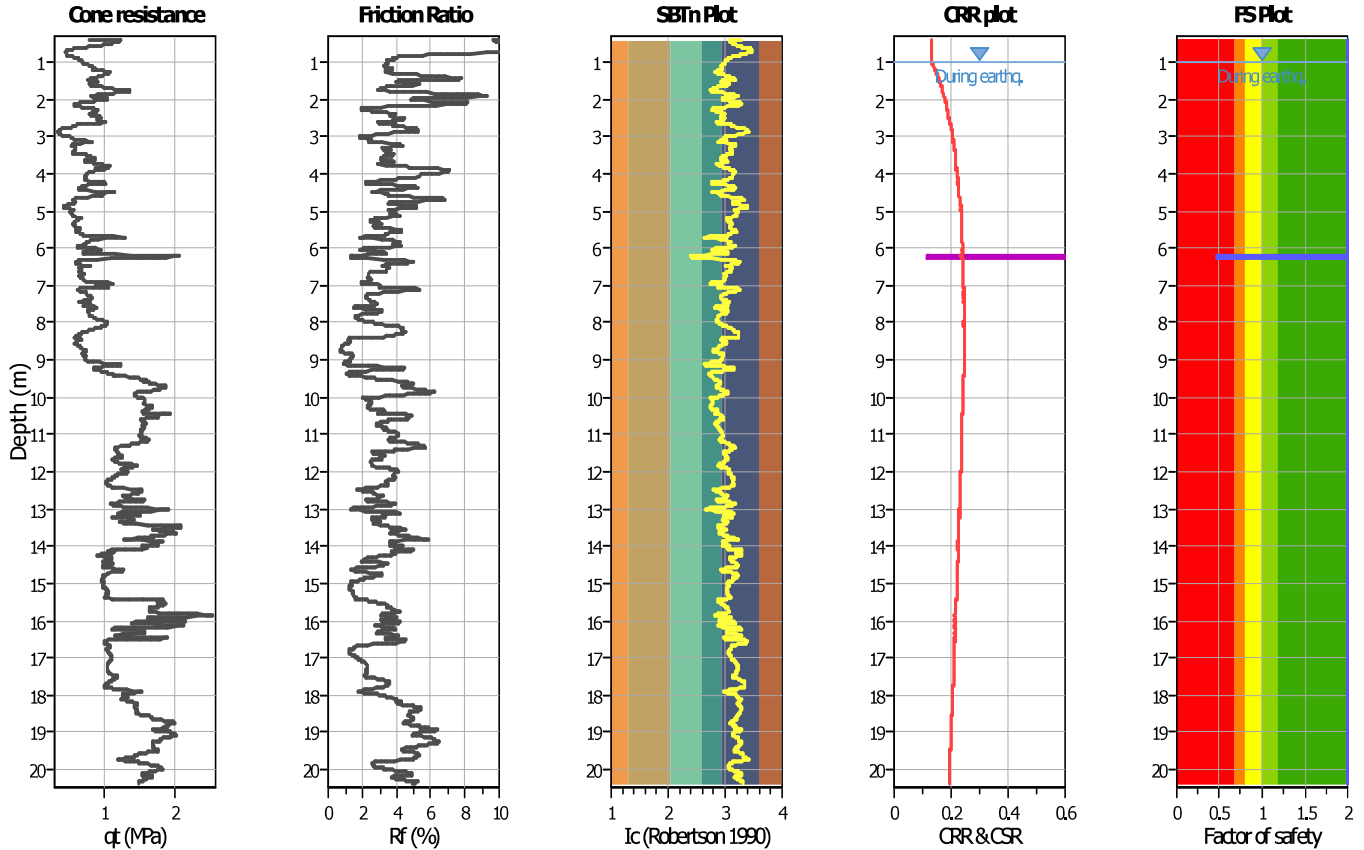
Project title :

Location : Campogalliano, TRANSMEC

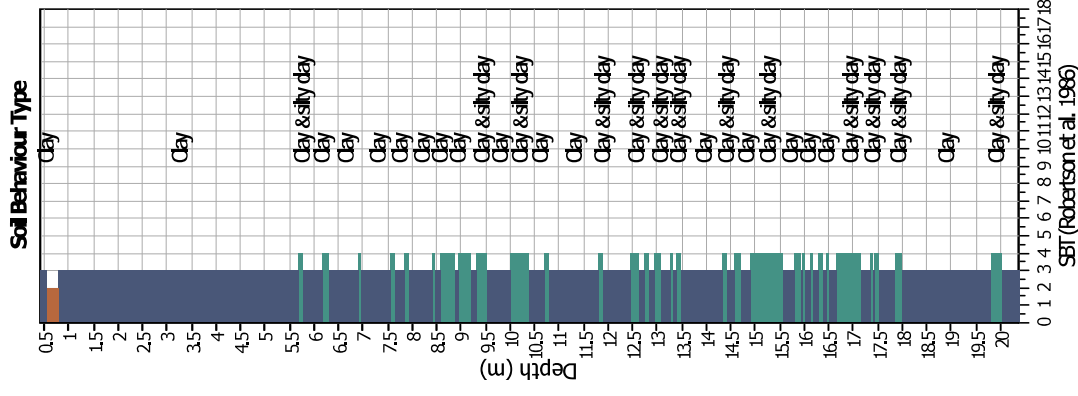
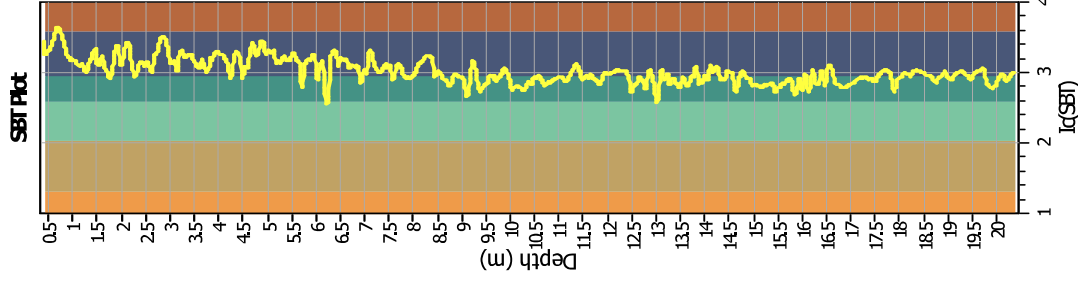
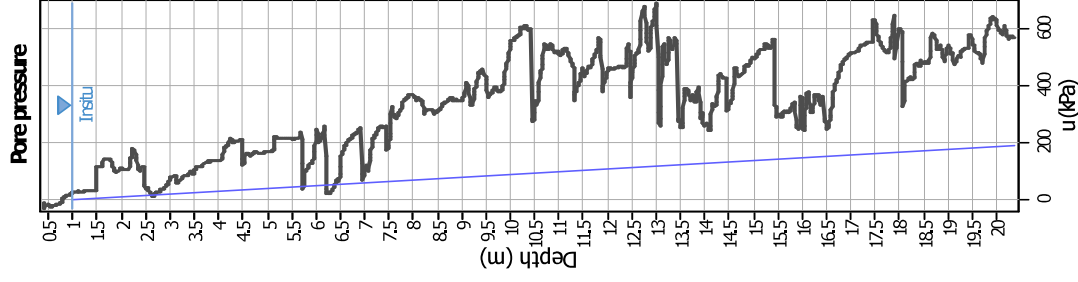
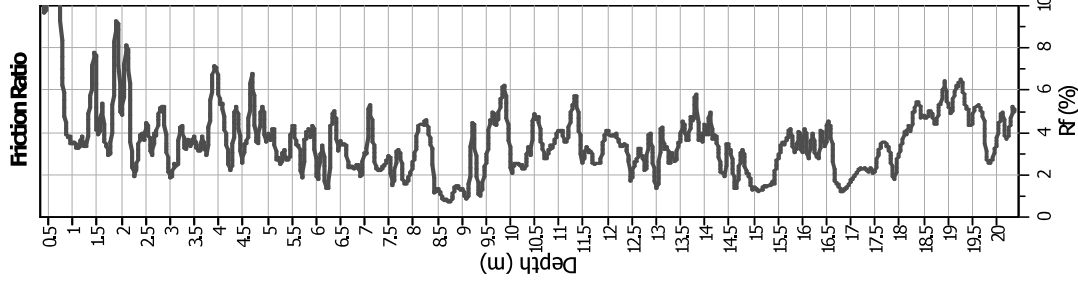
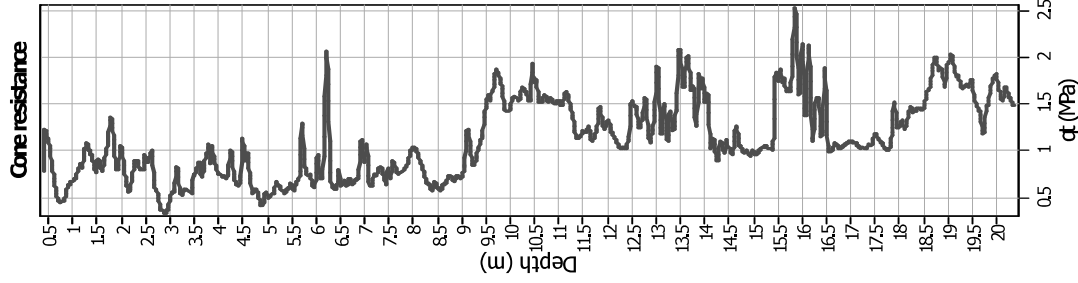
CPT file : CPTU 2

Input parameters and analysis data

Analysis method:	B&I (2014)	G.W.T. (in-situ):	1.00 m	Use fill:	No	Clay like behavior applied:	Sands only
Fines correction method:	B&I (2014)	G.W.T. (earthq.):	1.00 m	Fill height:	N/A	Limit depth applied:	Yes
Points to test:	Based on Ic value	Average results interval:	3	Fill weight:	N/A	Limit depth:	20.00 m
Earthquake magnitude M_w :	6.14	Ic cut-off value:	2.60	Trans. detect. applied:	No	MSF method:	Method
Peak ground acceleration:	0.24	Unit weight calculation:	Based on SBT	K_σ applied:	Yes		



CPT basic interpretation plots



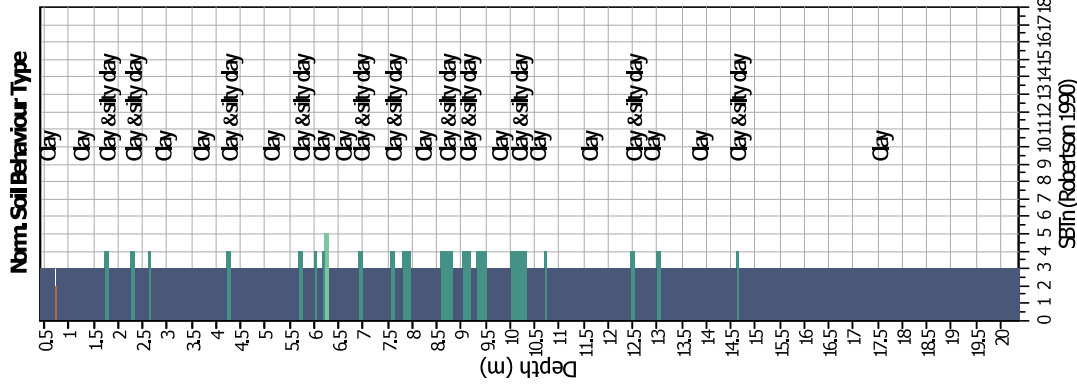
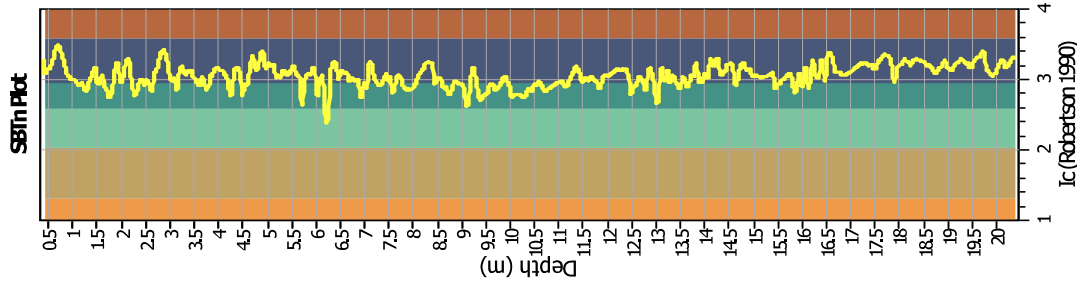
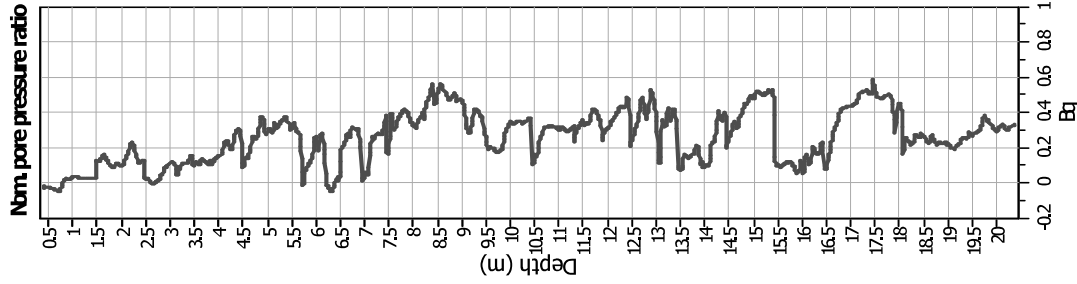
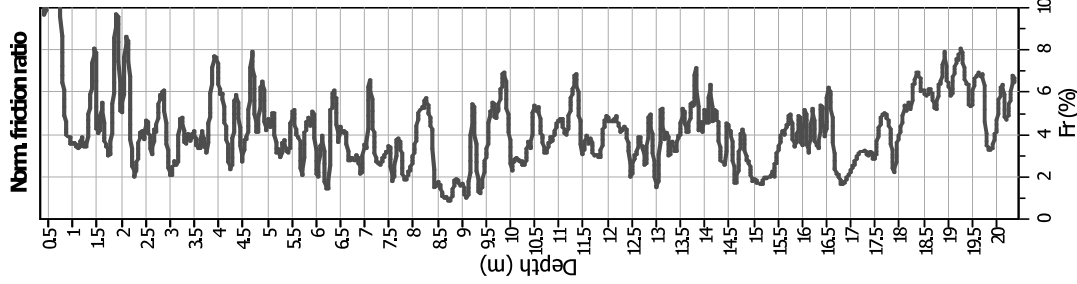
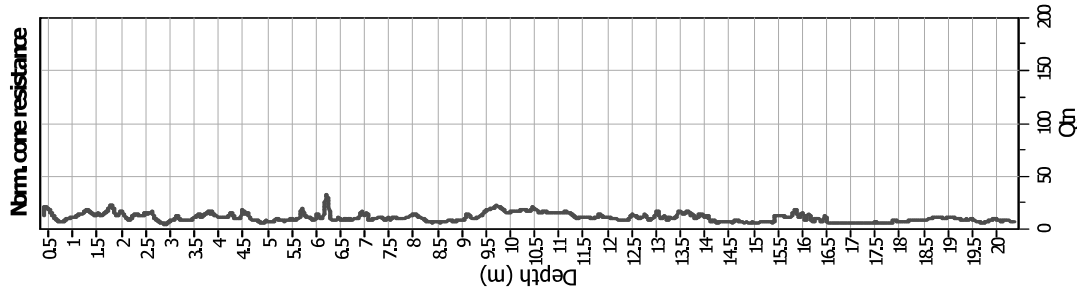
Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	1.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	No
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_p applied:	Yes
Earthquake magnitude M_w :	6.14	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sands only
Peak ground acceleration:	0.24	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	1.00 m	Fill height:	N/A	Limit depth:	20.00 m

SBT legend

1. Sensitive fine grained	4. Clayey silt to silty	7. Gravely sand to sand
2. Organic material	5. Silty sand to sandy silt	8. Very stiff sand to
3. Clay to silty clay	6. Clean sand to silty sand	9. Very stiff fine grained

CPT basic interpretation plots (normalized)



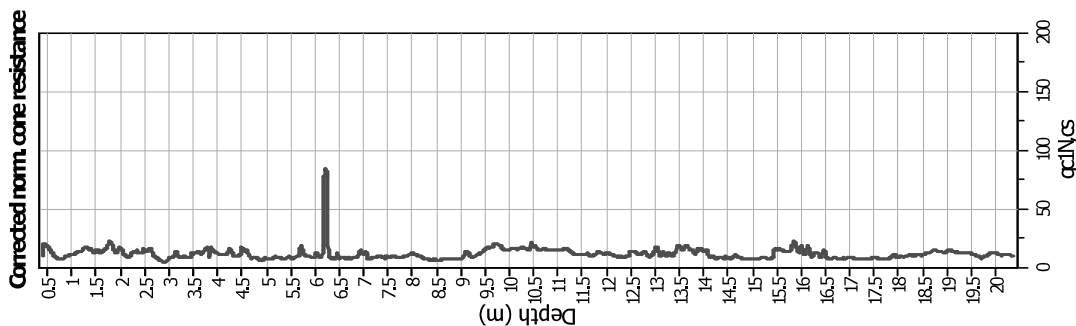
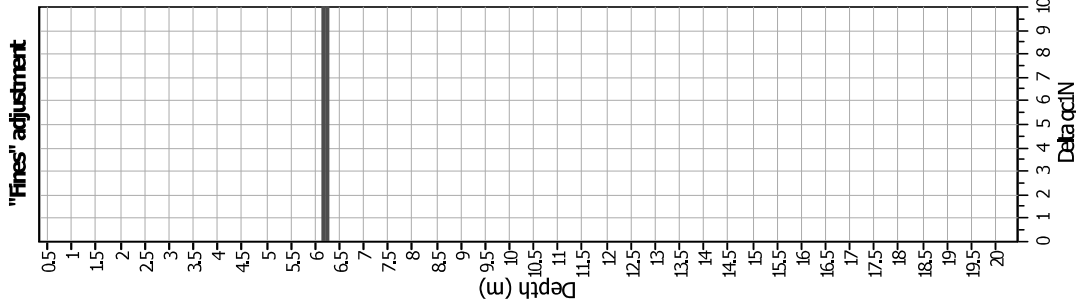
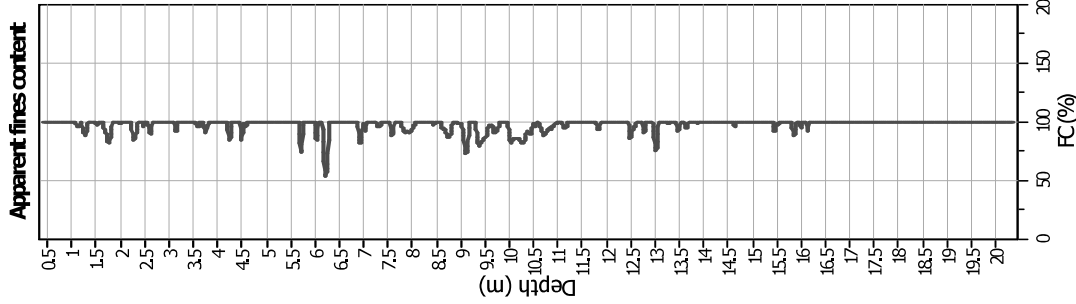
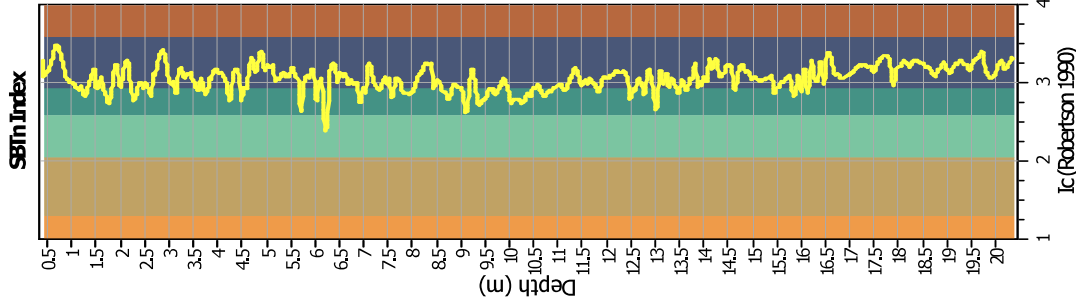
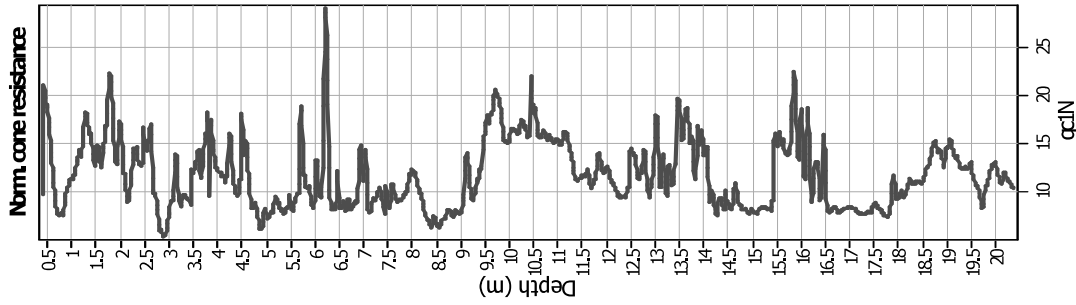
Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	1.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	No
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_p applied:	Yes
Earthquake magnitude M_w :	6.14	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sands only
Peak ground acceleration:	0.24	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	1.00 m	Fill height:	N/A	Limit depth:	20.00 m

SBTn legend

- 1. Sensitive fine grained
- 2. Organic material
- 3. Clay to silty clay
- 4. Clayey silt to silty
- 5. Silty sand to sandy silt
- 6. Clean sand to silty sand
- 7. Gravely sand to sand
- 8. Very stiff sand to
- 9. Very stiff fine grained

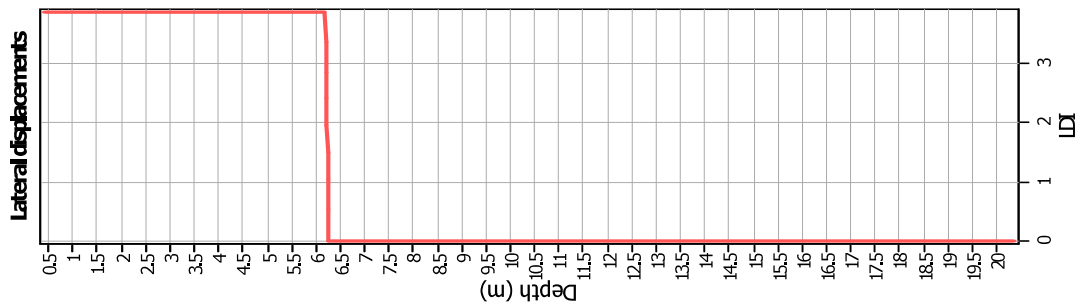
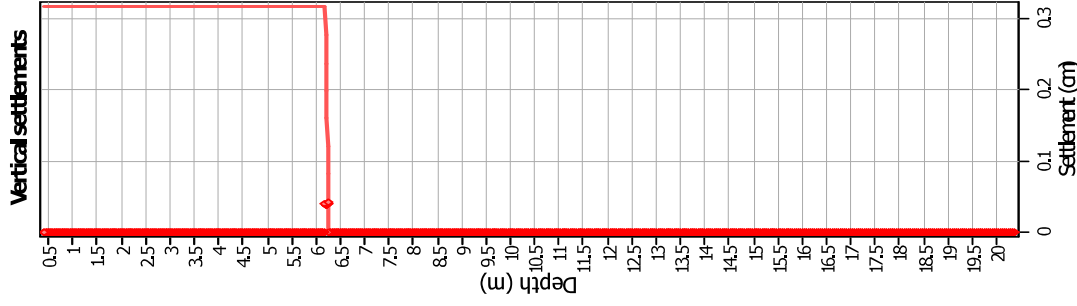
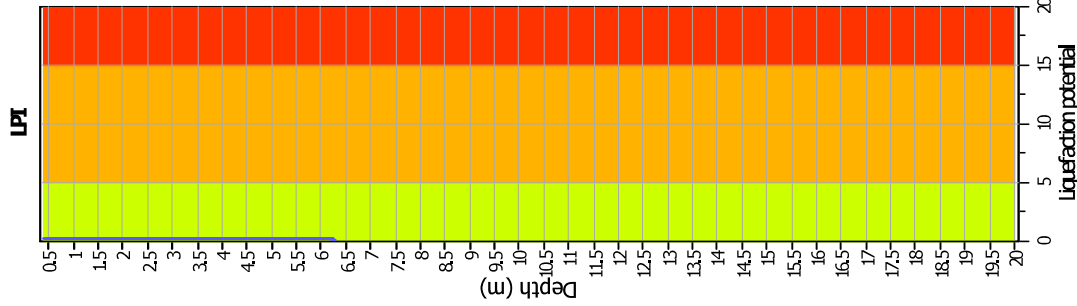
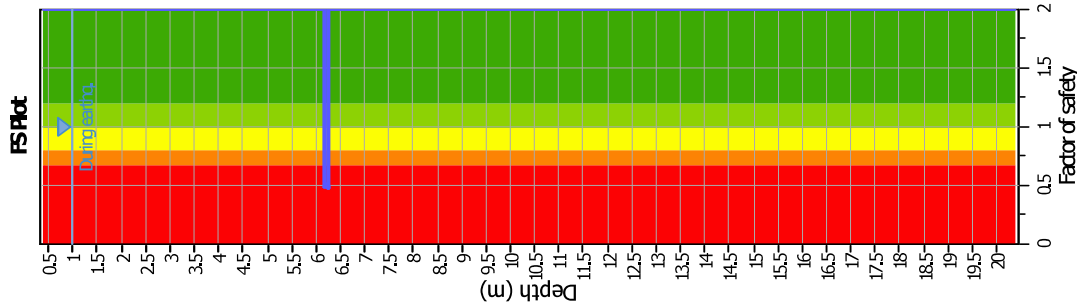
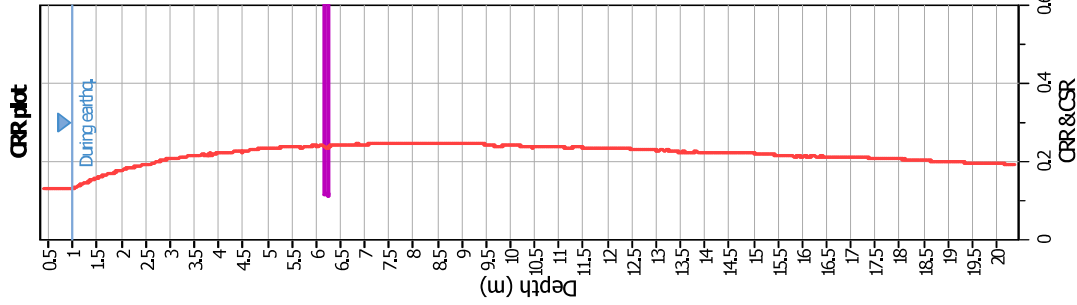
Liquefaction analysis overall plots (intermediate results)



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	1.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	No
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _r applied:	Yes
Earthquake magnitude M _w :	6.14	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sands only
Peak ground acceleration:	0.24	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	1.00 m	Fill height:	N/A	Limit depth:	20.00 m

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	1.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	No
Points to test:	Based on I _c value	I _c cut-off value:	2.60	K _σ applied:	Yes
Earthquake magnitude M _w :	6.14	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sands only
Peak ground acceleration:	0.24	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	1.00 m	Fill height:	N/A	Limit depth:	20.00 m

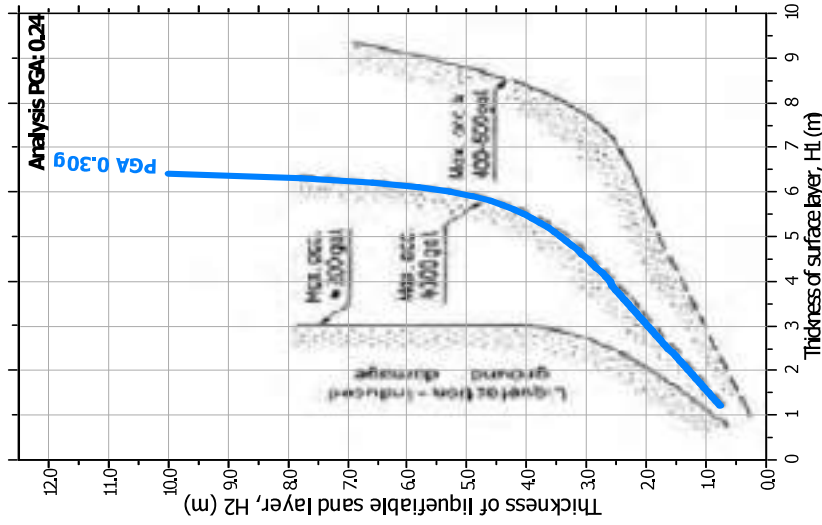
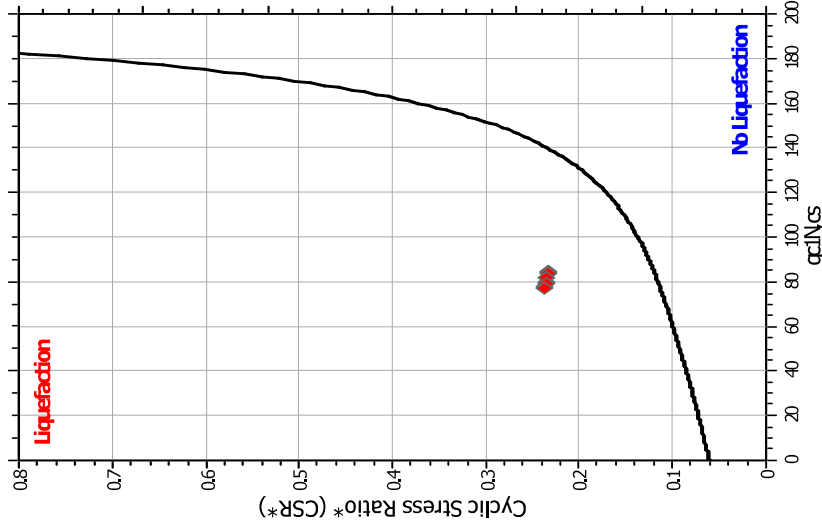
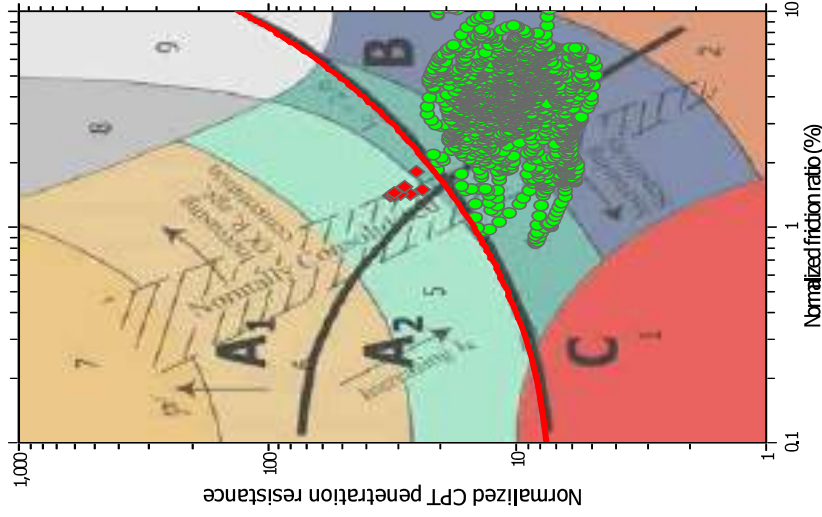
F.S. color scheme

■	Almost certain it will liquefy
■	Very likely to liquefy
■	Liquefaction and no liq. are equally likely
■	Unlike to liquefy
■	Almost certain it will not liquefy

LPI color scheme

■	Very high risk
■	High risk
■	Low risk

Liquefaction analysis summary plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	1.00 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	No
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K_r applied:	Yes
Earthquake magnitude M_w :	6.14	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sands only
Peak ground acceleration:	0.24	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	1.00 m	Fill height:	N/A	Limit depth:	20.00 m



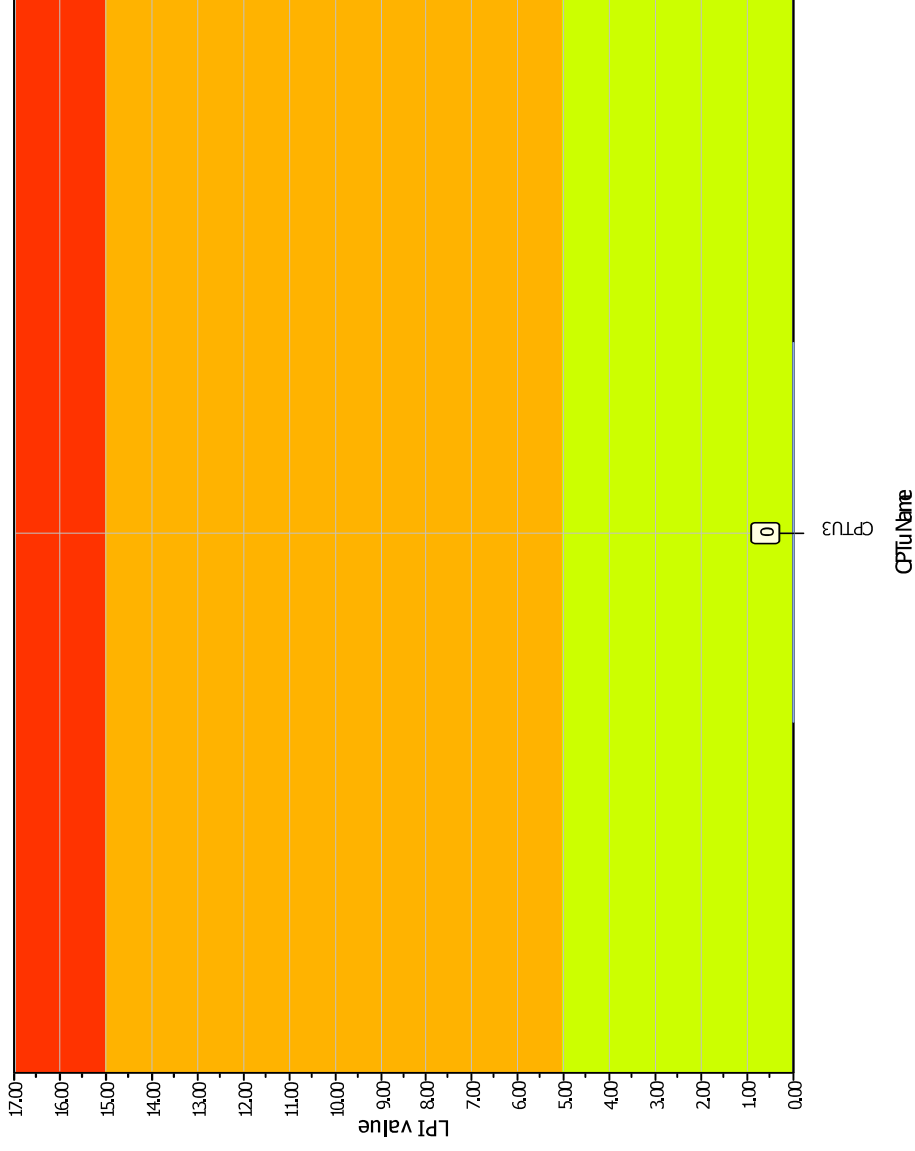
UFFICI: via Per Modena, 12 - 41051 Castelnuovo R. (MO) SEDE LEGALE: via C. Costa, 182 - 41123 Modena (MO)
tel. 059/3967169; 059/828367 - FAX 059/5960176 p. Iva e C.F. 02981500362
Email: info@geogroupmodena.it Pec: geo.group@winpec.it Sito: www.geogroupmodena.it
Operatore APR certificato ENAC per operazioni non critiche – Rif. ENAC 18627

Verifica alla liquefazione su CPTu 3

Project title : Studio del terreno di fondazione

Location : Campogalliano, TRANSMEC

Overall Liquefaction Potential Index report



LPI color scheme

- Very high risk
- High risk
- Low risk

Basic statistics

Total CPT number: 1
 100.00% low risk
 0.00% high risk
 0.00% very high risk

LIQUEFACTION ANALYSIS REPORT

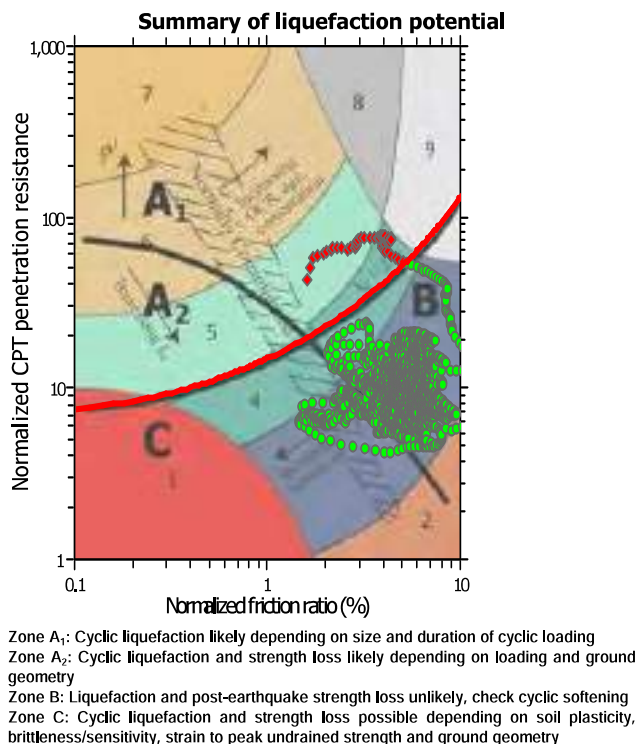
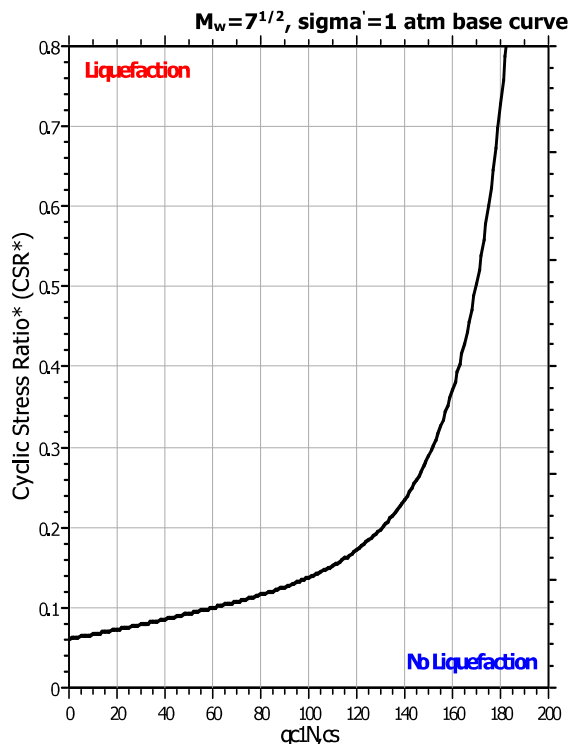
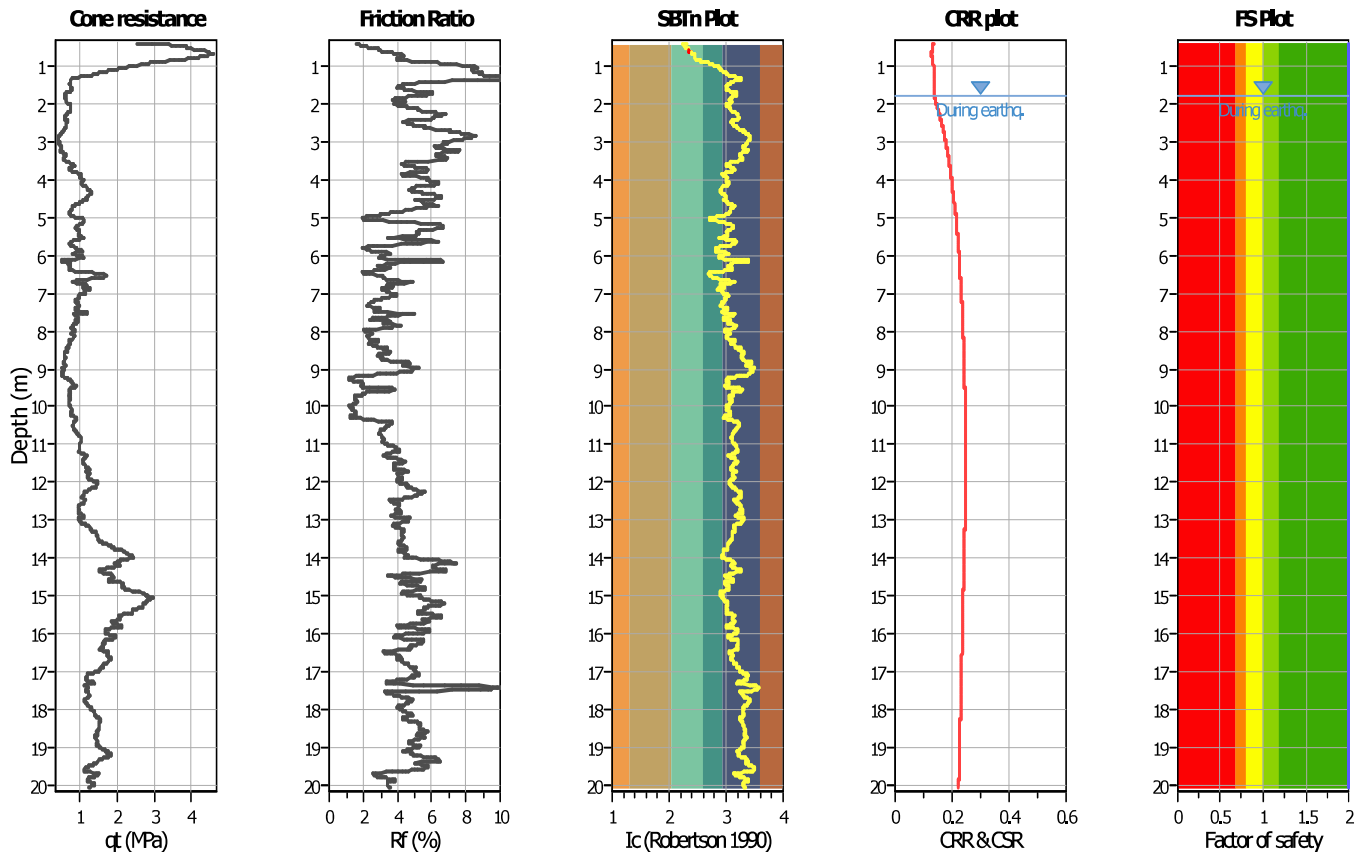
Project title : Studio del terreno di fondazione

Location : Campogalliano, TRANSMEC

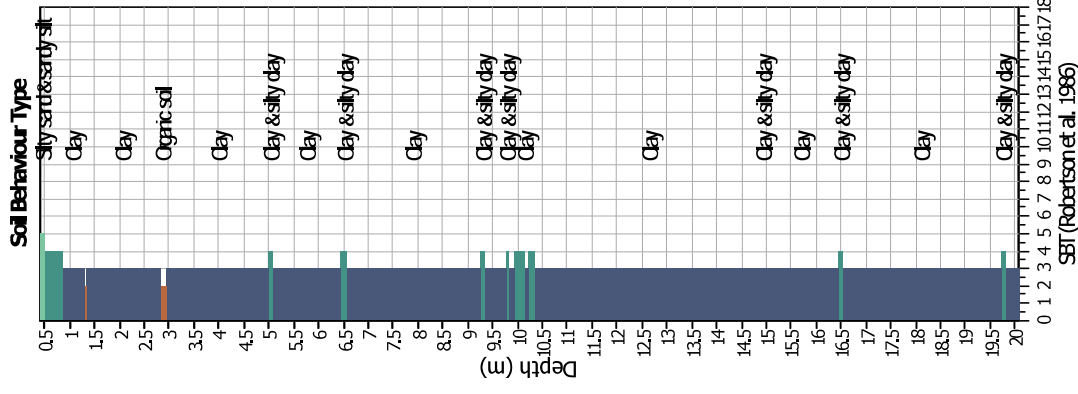
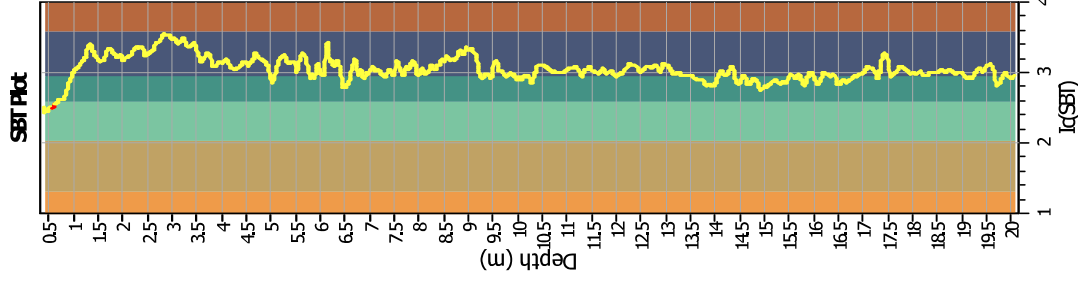
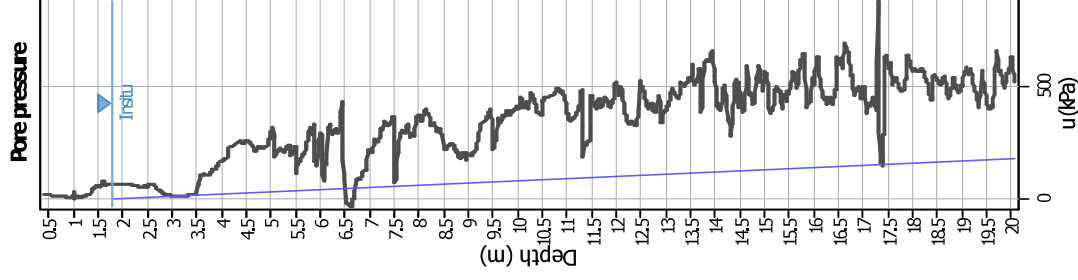
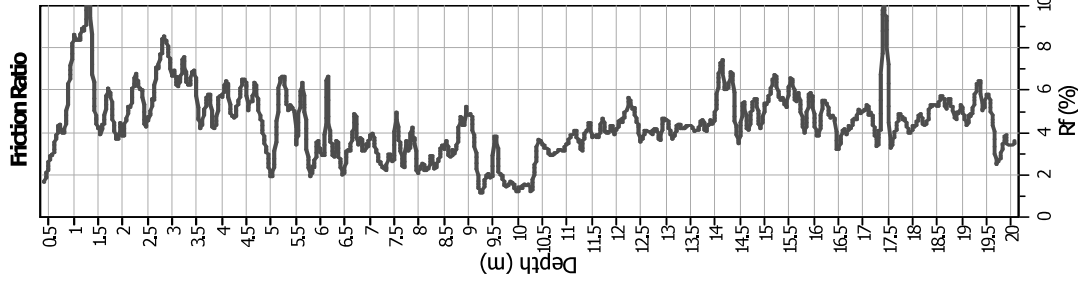
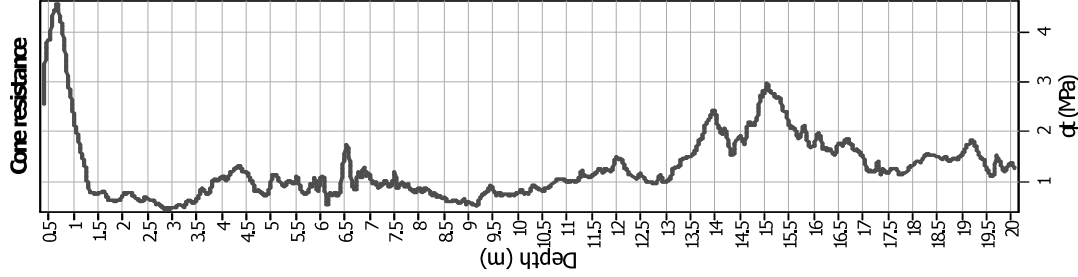
CPT file : CPTU3

Input parameters and analysis data

Analysis method:	B&I (2014)	G.W.T. (in-situ):	1.80 m	Use fill:	No	Clay like behavior applied:	Sands only
Fines correction method:	B&I (2014)	G.W.T. (earthq.):	1.80 m	Fill height:	N/A	Limit depth applied:	Yes
Points to test:	Based on Ic value	Average results interval:	3	Fill weight:	N/A	Limit depth:	20.00 m
Earthquake magnitude M_w :	7.00	Ic cut-off value:	2.60	Trans. detect. applied:	Yes	MSF method:	Method
Peak ground acceleration:	0.24	Unit weight calculation:	Based on SBT	K_σ applied:	Yes		



CPT basic interpretation plots



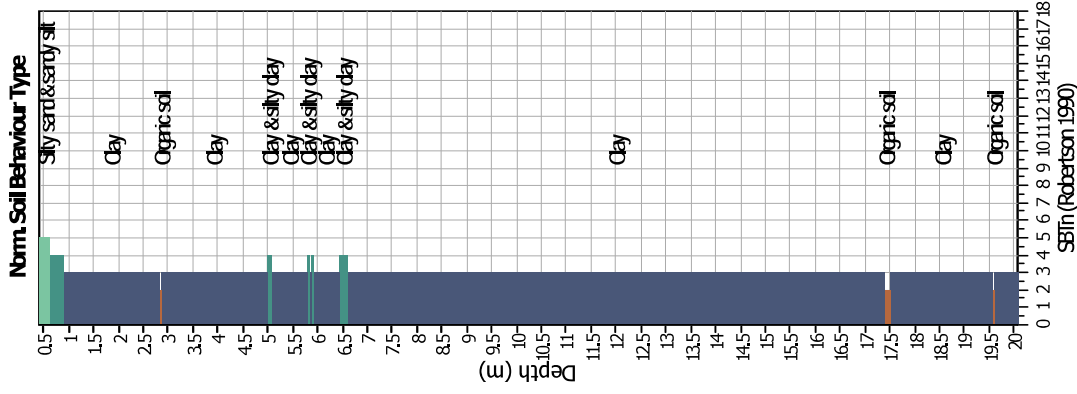
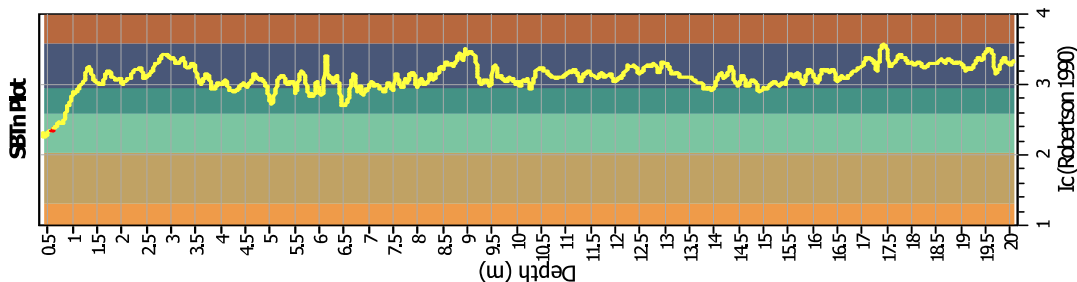
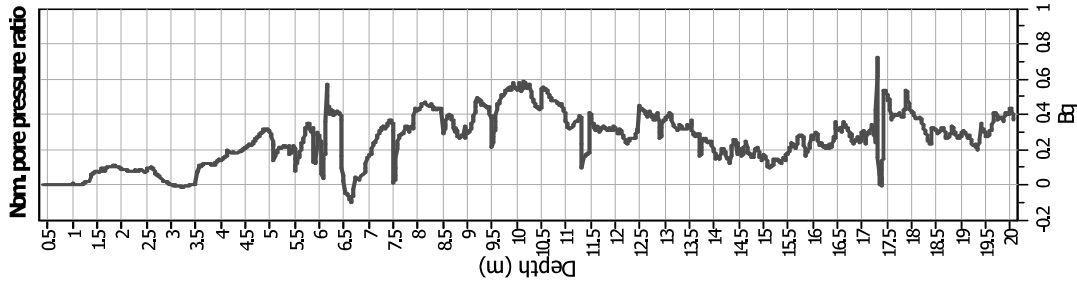
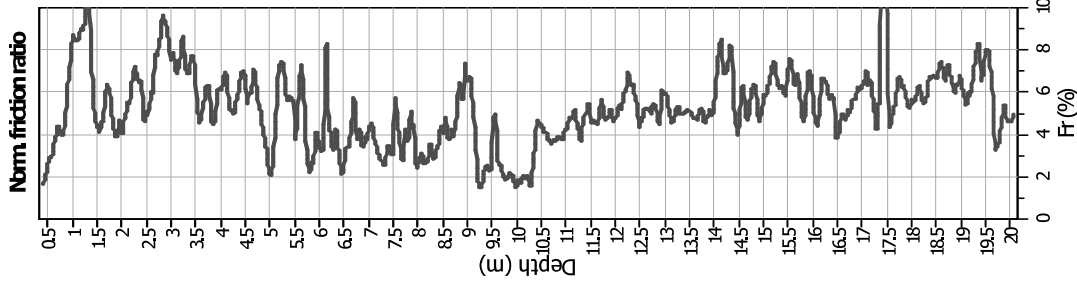
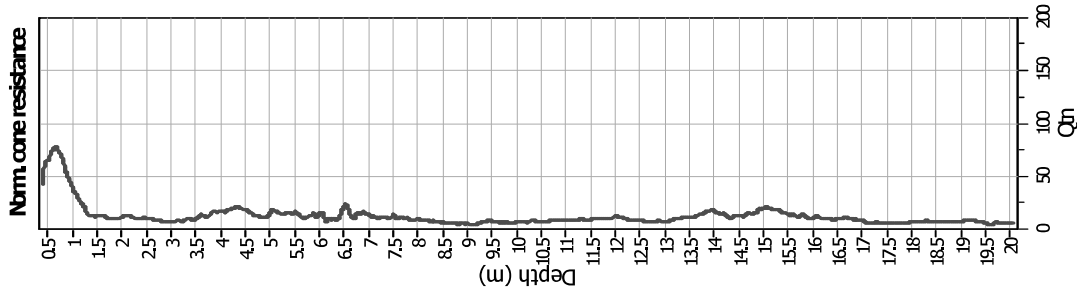
Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	1.80 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _v applied:	Yes
Earthquake magnitude M _w :	7.00	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sands only
Peak ground acceleration:	0.24	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	1.80 m	Fill height:	N/A	Limit depth:	20.00 m

SBT legend

- 1. Sensitive fine grained
- 2. Organic material
- 3. Clay to silty clay
- 4. Clayey silt to silty
- 5. Silty sand to sandy silt
- 6. Clean sand to silty sand
- 7. Gravely sand to sand
- 8. Very stiff sand to
- 9. Very stiff fine grained

CPT basic interpretation plots (normalized)



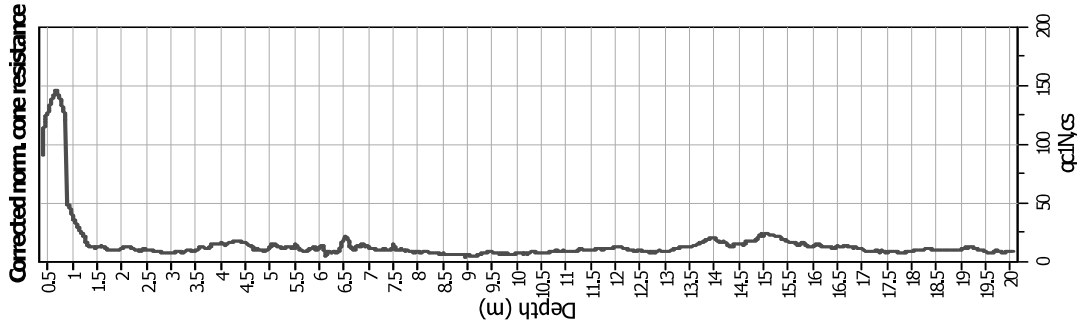
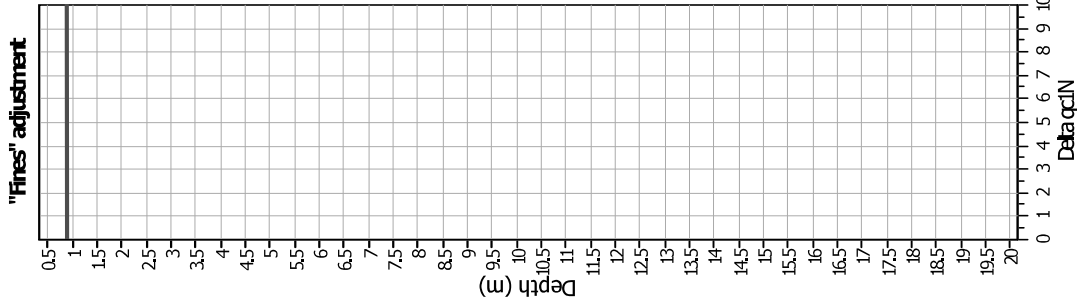
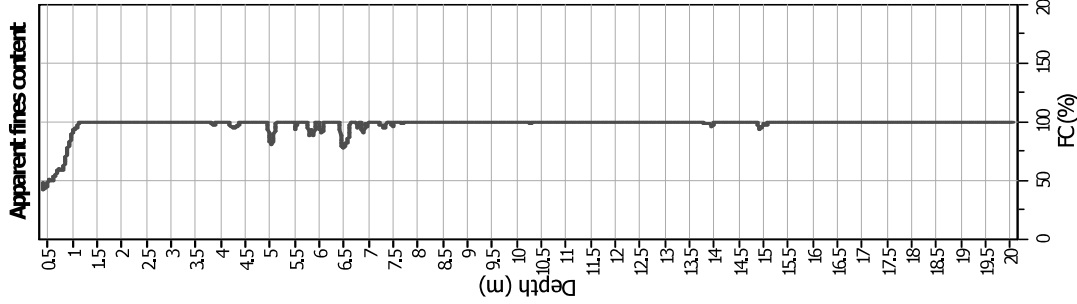
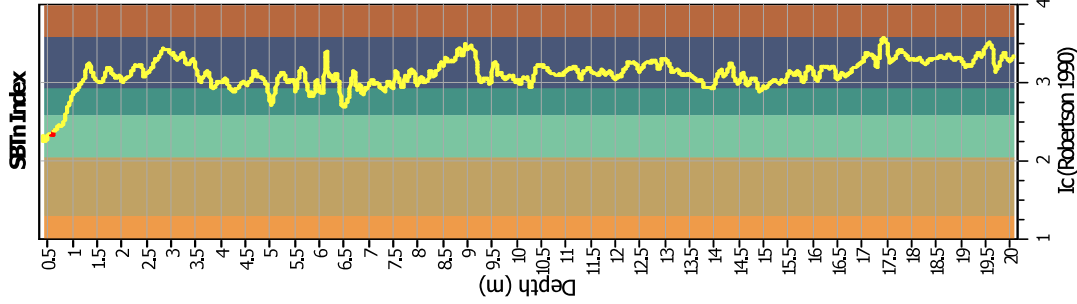
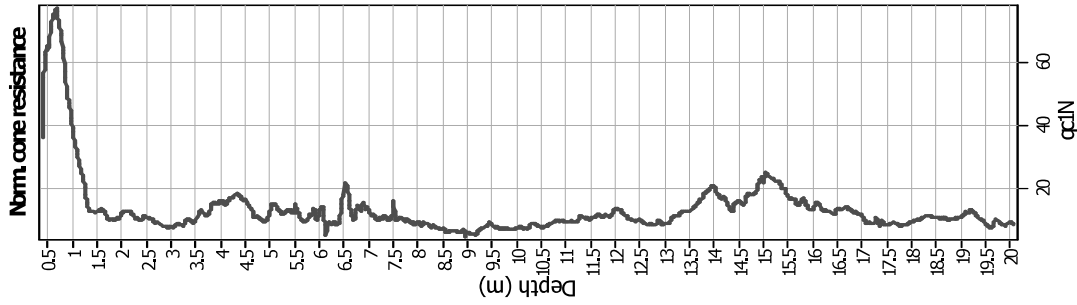
Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	1.80 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _v applied:	Yes
Earthquake magnitude M _w :	7.00	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sands only
Peak ground acceleration:	0.24	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	1.80 m	Fill height:	N/A	Limit depth:	20.00 m

SBTn legend

- 1. Sensitive fine grained
- 2. Organic material
- 3. Clay to silty clay
- 4. Clayey silt to silty
- 5. Silty sand to sandy silt
- 6. Clean sand to silty sand
- 7. Gravely sand to sand
- 8. Very stiff sand to
- 9. Very stiff fine grained

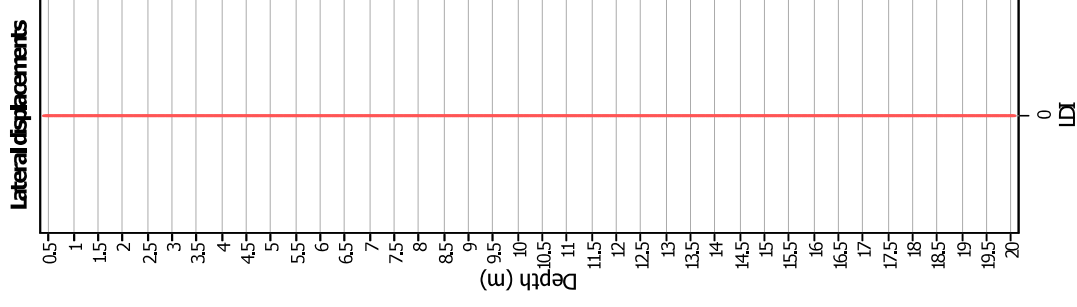
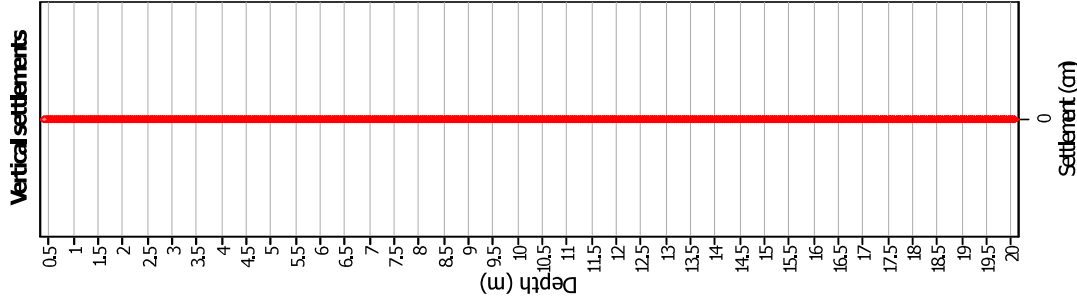
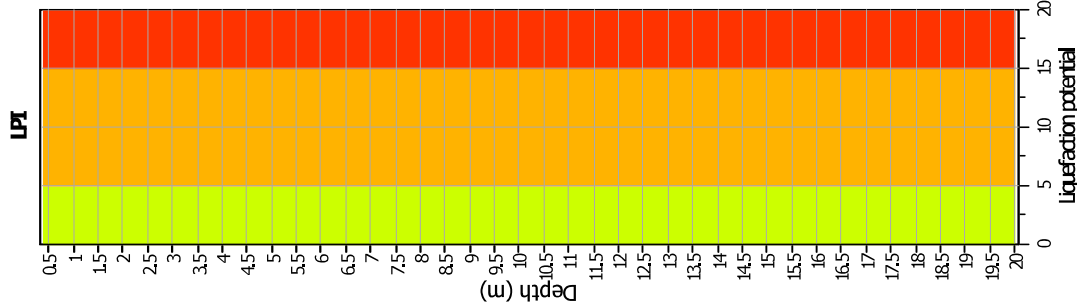
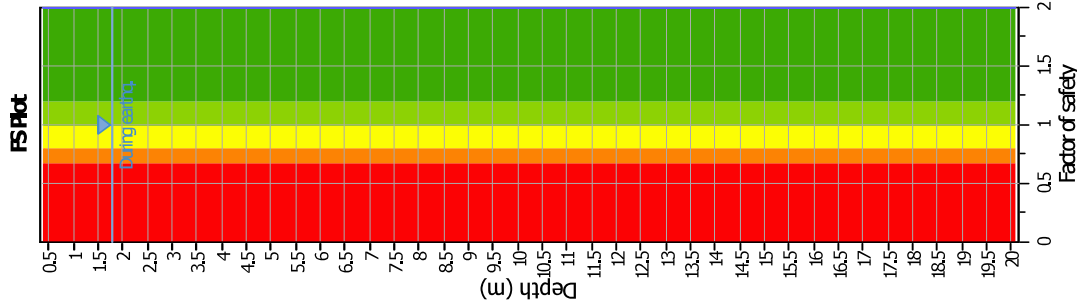
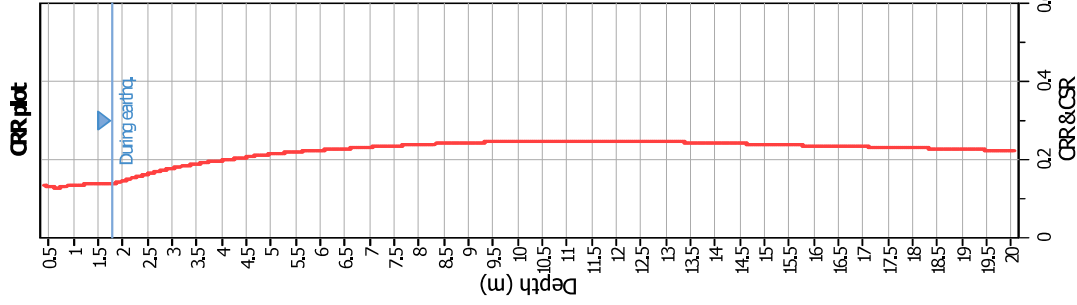
Liquefaction analysis overall plots (intermediate results)



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	1.80 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _r applied:	Yes
Earthquake magnitude M _w :	7.00	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sands only
Peak ground acceleration:	0.24	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	1.80 m	Fill height:	N/A	Limit depth:	20.00 m

Liquefaction analysis overall plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	1.80 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _c applied:	Yes
Earthquake magnitude M _w :	7.00	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sands only
Peak ground acceleration:	0.24	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	1.80 m	Fill height:	N/A	Limit depth:	20.00 m

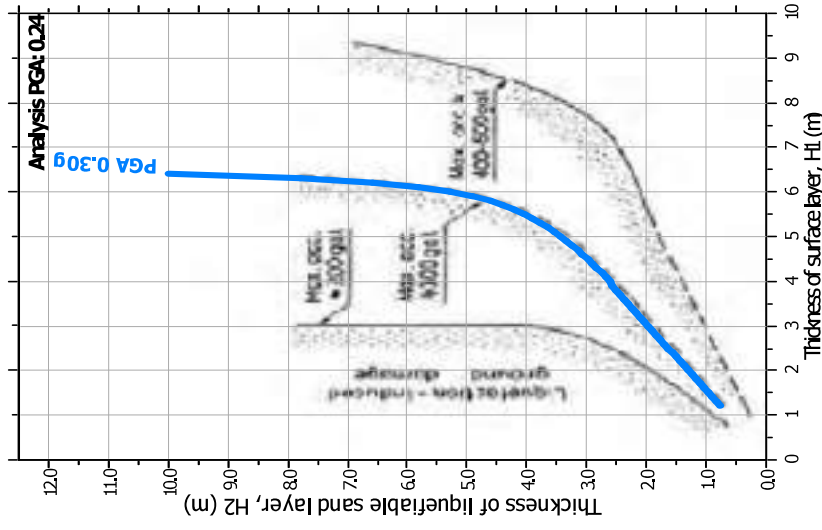
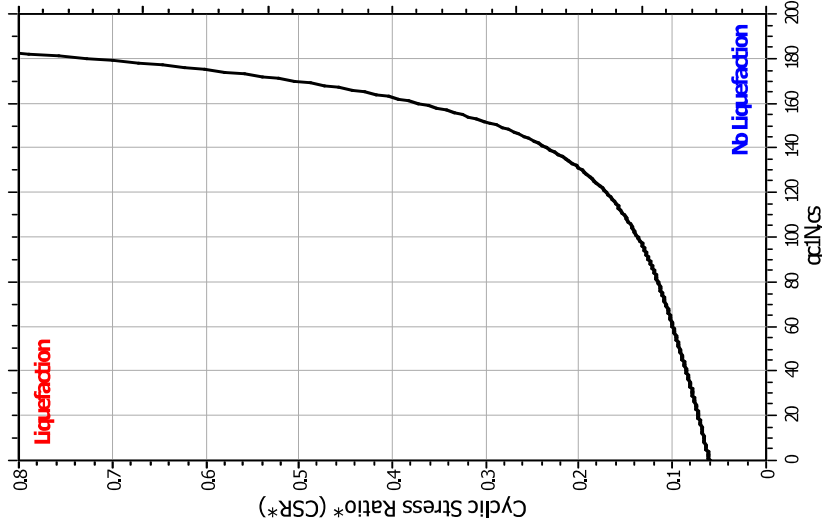
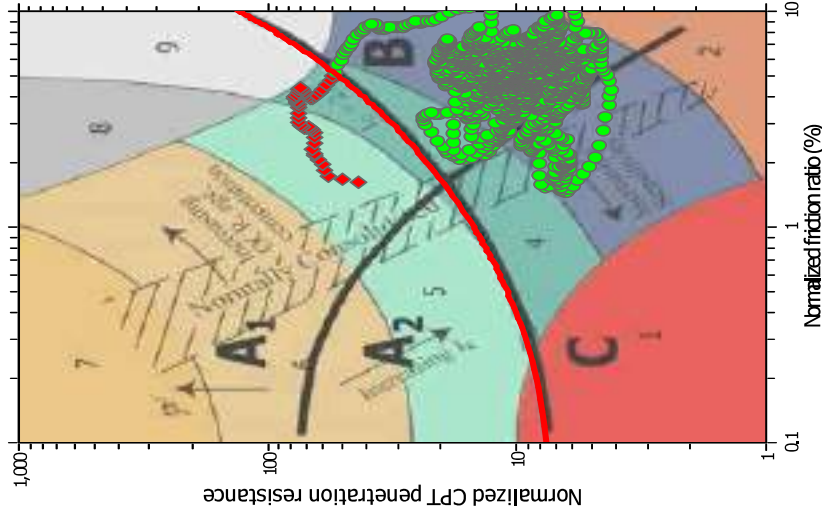
F.S. color scheme

- Almost certain it will liquefy
- Very likely to liquefy
- Liquefaction and no liq. are equally likely
- Unlikely to liquefy
- Almost certain it will not liquefy

LPI color scheme

- Very high risk
- High risk
- Low risk

Liquefaction analysis summary plots



Input parameters and analysis data

Analysis method:	B&I (2014)	Depth to GWT (earthq.):	1.80 m	Fill weight:	N/A
Fines correction method:	B&I (2014)	Average results interval:	3	Transition detect. applied:	Yes
Points to test:	Based on Ic value	Ic cut-off value:	2.60	K _r applied:	Yes
Earthquake magnitude M _w :	7.00	Unit weight calculation:	Based on SBT	Clay like behavior applied:	Sands only
Peak ground acceleration:	0.24	Use fill:	No	Limit depth applied:	Yes
Depth to water table (insitu):	1.80 m	Fill height:	N/A	Limit depth:	20.00 m